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ABBREVIATIONS AND SYMBOLS

A/E	ARCHITECT/ENGINEER	FDN	FOUNDATION	PAF	POWDER ACTUATED FASTENER
ABV	ABOVE	FF	FINISHED FLOOR	PRL	PARALLEL
ADDL	ADDITIONAL	FL	FLOOR	PCF	POUNDS PER CUBIC FOOT
ADJ	ADJACENT	FP	FIREPROOF(ING)	PCI	POUNDS PER CUBIC INCH
AFF	ABOVE FINISHED FLOOR	FS	FAR SIDE	PEMB	PRE-ENGINEERED METAL BUILDING
AHU	AIR HANDLING UNIT	FT	FOOT/FEET	PERM	PERIMETER
ALT	ALTERNATE	FTG	FOOTING	PERP	PERPENDICULAR
APPROX	APPROXIMATE(LY)	GA	GAUGE/GAGE	PJP	PARTIAL JOINT PENETRATION
ARCH	ARCHITECT(URAL)	GALV	GALVANIZED	PL	PLATE
AT	ANTITERRORISM	GB	GRADE BEAM	PLF	POUNDS PER LINEAR FOOT
AVG	AVERAGE	GC	GENERAL CONTRACTOR	PC	PRECAST
AWTS	AUTOMATIC WELDED THREADED STUDS	HORIZ	HORIZONTAL	PREFAB	PREFABRICATED
B PL	BASE PLATE OR BEARING PLATE	HP	HIGH POINT	PSF	POUNDS PER SQUARE FOOT
B/	BOTTOM OF	HSA	HEADED STUD ANCHOR	PSI	POUNDS PER SQUARE INCH
BD	BOARD	HT	HEIGHT	PT	PRE/POST-TENSIONING
BF	BRACED FRAME	I/F	INSIDE FACE	PTW	PRESSURE TREATED WOOD
BFF	BELOW FINISHED FLOOR	ID	INSIDE DIAMETER	PVMT	PAVEMENT
BLDG	BUILDING	IN	INCH(ES)	QTY	QUANTITY
BLK	BLOCK(ING)	INCL	INCLUDE	RAD	RADIUS
BLW	BELOW	INFO	INFORMATION	RE:	REFER TO
BM	BEAM	INT	INTERIOR	REINF	REINFORCEMENT
BOT	BOTTOM	ISO JT	ISOLATION JOINT	REQD	REQUIRED
BRG	BEARING	JST	JOIST	REV	REVISE(ION)
BS	BOTH SIDES	JT	JOINT	RO	ROUGH OPENING
BTWN	BETWEEN	K	KIP(S)	RTU	ROOF TOP UNIT
CC	CENTER TO CENTER	KB	KNEE BRACE	SC	SLIP CRITICAL
CF	CUBIC FOOT OR CUBIC FEET	KCF	KIPS PER CUBIC FEET	SCHED	SCHEDULE
CFMF	COLD-FORMED METAL FRAMING	KLF	KIPS PER LINEAR FOOT	SECT	SECTION
CIP	CAST IN PLACE	KSF	KIPS PER SQUARE FEET	SF	SQUARE FOOT
CJ	CONTROL JOINT/CONSTRUCTION JOINT	KSI	KIPS PER SQUARE INCH	SHT	SHEET
CJP	COMPLETE JOINT PENETRATION	L	LENGTH	SIM	SIMILAR
CL	CENTERLINE	LAT	LATERAL	SL	SLOPE(D) OR SLOPING
CLR	CLEAR OR CLEAR COVER	LBS	POUNDS	SLV	SLEEVE
CMU	CONCRETE MASONRY UNIT	ld	DEVELOPMENT LENGTH	SOG	SLAB ON GRADE
COL	COLUMN	Ldh	HOOK DEVELOPMENT LENGTH	SOD	SLAB ON METAL DECK
CONC	CONCRETE	Lst	LAP SPlice LENGTH	SP	SPACE(S) OR SPACING
CONN	CONNECTION	Lsc	LAP SPlice LENGTH	SPEC	SPECIFY OR SPECIFICATIONS
CONST	CONSTRUCTION	LF	LINEAR FOOT	SQ	SQUARE
CONT	CONTINUOUS	LL	LIVE LOAD	SS	STAINLESS STEEL
CONTR	CONTRACTOR	LLH	LONG LEG HORIZONTAL	STD	STANDARD
COORD	COORDINATE	LLV	LONG LEG VERTICAL	STIFF	STIFFENER
CTR	CENTER(ED)	LONG	LONGITUDINAL	STL	STEEL
CY	CUBIC YARD	LP	LOW POINT	STRUCT	STRUCTURAL
db	BAR DIAMETER	LSH	LONG SIDE HORIZONTAL	SUSP	SUSPEND(ED) OR SUSPENSION
DBA	DEFORMED BAR ANCHOR	LSV	LONG SIDE VERTICAL	T&B	TOP AND BOTTOM
DBL	DOUBLE	LWT	LIGHT WEIGHT	T/	TOP OF
DET	DETAIL	MEP	MECHANICAL, ELECTRICAL, & PLUMBING	TEMP	TEMPORARY
DIA	DIAMETER	MATL	MATERIAL	THD	THREAD(ED)
DIAG	DIAGONAL	MAX	MAXIMUM	THK	THICK(NESS)
DIM	DIMENSION	MCJ	MASONRY CONTROL JOINT	TL	TOTAL LOAD
DL	DEAD LOAD	MECH	MECHANICAL	TRANS	TRANSVERSE
DN	DOWN	MEZZ	MEZZANINE	TRTD	TREATED
DTL	DETAIL	MFR	MANUFACTURE(R)	TYP	TYPICAL
DWG	DRAWING	MID	MIDDLE	UNO	UNLESS NOTED OTHERWISE
DWL	DOWEL	MIN	MINIMUM	VERT	VERTICAL
E/	EDGE OF	MISC	MISCELLANEOUS	VIF	VERIFY IN FIELD
EA	EACH	MULT	MULTIPLE	W	WIDTH
EF	EACH FACE	MO	MASONRY OPENING	W/	WITH
EIFS	EXTERIOR INSULATION FINISH SYSTEM	MTL	METAL	W/C	WATER TO CEMENT RATIO
EJ	EXPANSION JOINT	MWT	MEDIUM WEIGHT	W/O	WITHOUT
ELEC	ELECTRICAL	NF	NEAR FACE	WL	WIND LOAD
ELEV	ELEVATION(S)	NIC	NOT IN CONTRACT	WP	WORKING POINT
EMBED	EMBED(ED)(MENT)	NUM	NUMBER	WT	WEIGHT
ENG	ENGINEER	NOM	NOMINAL	WWR	WELDED WIRE REINFORCEMENT
EOR	ENGINEER OF RECORD	NS	NEAR SIDE		
EQ	EQUAL	NTS	NOT TO SCALE	@	AT / AT EACH
EQUIP	EQUIPMENT	NWT	NORMAL WEIGHT	()°	DEGREE
EST	ESTIMATED	OIF	OUTSIDE FACE	ø	DIAMETER
EW	EACH WAY	OC	ON CENTER	#	NUMBER
EXCL	EXCLUDE(ING)	OD	OUTSIDE DIAMETER		
(E)	EXISTING	OPNG	OPENING	o FD	FLOOR DRAIN
EXP	EXPANSION	OPP	OPPOSITE	o RD	ROOF DRAIN
EXT	EXTERIOR	OH	OPPOSITE HAND		
F/	FACE OF	OVH	OVERHEAD		
F/F	FACE TO FACE	OWJ	OPEN WEB STEEL JOIST		

DRAWING LEGEND		
GENERAL ANNOTATIONS	CONCRETE CONSTRUCTION	STEEL CONSTRUCTION
FS# CONC SPREAD FTG TAG	CONC SPREAD FOOTING	STEEL COLUMN (W SHAPES)
FC# CONC CONTINUOUS FTG TAG	CONC CONTINUOUS FOOTING	STEEL COLUMN (HSS)
XC# COLUMN TAG	CONC COLUMN	STEEL COLUMN (HSS ROUND)
XW# WALL TAG	CONC WALL	STEEL BEAM / GIRDER
XB# BEAM TAG	CONC BEAM	STEEL GIRDER TRUSS
XP# PIER TAG	CONC PIER	STEEL TRUSS JOIST
'X' = MATERIAL C = CONCRETE M = MASONRY S = STEEL W = WOOD	FOUNDATION PEDESTAL	DRAG STRUT CONNECTION
# = NUMERICAL DESIGNATION	CONC COLUMN BELOW	FULLY RESTRAINED MOMENT CONNECTION
REF ELEVATION CALLOUT (SECTION / DETAILS)	CONC BEAM	PARTIALLY RESTRAINED MOMENT CONNECTION
REF ELEVATION CALLOUT (PLAN)	CONC BEAM/WALL BELOW	BRACED FRAME (RE: STRUCTURAL ELEVATIONS)
REF = T/OBJECT OR B/OBJECT XX' - YY" = OBJECT ELEVATION FROM DATUM	CONC LINTEL	SPLICE CONNECTION
CHANGE IN TOP OF ELEV	NOTE: AT FLOOR OR ROOF FRAMING PLANS, OPENINGS SHOWN ARE IN WALL BELOW	BEAM SIZE (X) C=Y"
SLOPE DESIGNATION (SEE ARCH FOR ACTUAL SLOPES)	REINFORCED CAST-IN-PLACE CONCRETE SUSPENDED SLAB	BEAM SIZE = BEAM DESIGNATION X = # OF HEADED STUDS (SPACED UNIFORMLY) Y = BEAM CAMBER (CROWN UPWARD @ MIDSPAN) = SPECIAL REACTIONS (kips) OR OTHER NOTES
START OF SLOPE WHERE SHOWN	CS # (1 WAY) CS # (2 WAY)	
PLAN REFERENCE	CONCRETE SLAB ON GRADE	
TYPICAL (TYP) OR SIMILAR (SIM) DETAIL	SOG #	
SHEET REFERENCE		
DETAIL, SECTION OR ELEVATION REFERENCE		
TYPICAL (TYP) OR SIMILAR (SIM) DETAIL		
SHEET REFERENCE		
GREY TONE DESIGNATES EXISTING CONSTRUCTION BLACK TONE DESIGNATES NEW CONSTRUCTION UNLESS NOTED OTHERWISE		
NEW CONST EXIST CONST		

DC-1 BUILDING CODE:

A. INTERNATIONAL BUILDING CODE (IBC) 2021 AS AMENDED BY
1. UFC 1-200-01 W/ CHANGE 3, DATED 26 FEB 2024
2. UFC 3-301-01 W/ CHANGE 1, DATED 11 APR 2023
B. EDITION OF ALL REFERENCED STANDARDS NOTED HEREIN ARE AS NOTED IN THE BUILDING CODE.

DC-2 VERTICAL LOADS

A. DEAD LOADS (INCLUDES SELF-WEIGHT)
1. ROOF 30 PSF
A. MINIMUM (FOR UPLIFT) 12 PSF
B. LIVE LOADS
1. ROOF (REDUCIBLE PER ASCE 7) 20 PSF MINIMUM
2. FLOORS (REDUCIBLE PER ASCE 7)
A. TYPICAL GROUND FLOOR 100 PSF
B. HANGARS 200 PSF
C. STORAGE 125 PSF
D. MECHANICAL 150 PSF
C. SNOW LOADS
1. GROUND SNOW LOAD (Pg) 5 PSF
2. ADDITIONAL SNOW DRIFT AND SLIDING SNOW AS PER APPLICABLE BUILDING CODE, REFER TO S-005.
D. CONSTRUCTION LOADS 20 PSF

DC-3 LATERAL LOADS

A. RISK CATEGORY III
B. WIND DESIGN CRITERIA
1. BASIC DESIGN WIND SPEED (V) 105 MPH
2. ALLOWABLE DESIGN WIND SPEED (V_{asd}) 82 MPH
3. EXPOSURE CATEGORY C
4. INTERNAL PRESSURE COEFFICIENT
A. PARTIALLY ENCLOSED (FULL WIND SPEED) +/- 0.55
5. COMPONENTS AND CLADDING RE: S-005
6. WIND ULTIMATE BASE SHEAR
A. PLAN EAST/WEST (AREA A,D) 101 K
B. PLAN EAST/WEST (AREA B,C) 125 K
C. PLAN NORTH/SOUTH (AREA A,D) 174 K
D. PLAN NORTH/SOUTH (AREA B,C) 187 K
C. SEISMIC DESIGN CRITERIA
1. SEISMIC IMPORTANCE FACTOR (I_s) 1.25
2. SITE CLASS D
3. MAPPED SPECTRAL RESPONSE ACCELERATION
A. SHORT PERIOD (S_s) 0.724
B. ONE SECOND (S₁) 0.226
4. DESIGN SPECTRAL RESPONSE ACCELERATION PARAMETERS
A. SHORT PERIOD (S_{os}) 0.589
B. ONE SECOND (S_{o1}) 0.324
5. SEISMIC DESIGN CATEGORY D
6. SEISMIC RESPONSE COEFFICIENT (C_s) 0.123
7. SEISMIC DESIGN BASE SHEAR
A. PLAN EAST/WEST (AREA A,D) 84 K
B. PLAN EAST/WEST (AREA B,C) 79 K
C. PLAN NORTH/SOUTH (AREA A,D) 84 K
D. PLAN NORTH/SOUTH (AREA B,C) 79 K
8. SEISMIC RESISTING SYSTEM:
A. STEEL SPECIAL CONCENTRIC BRACED FRAMES
1. RESPONSE MODIFICATION R = 6
2. DEFLECTION AMPLIFICATION C_o = 5
3. OVERSTRENGTH FACTOR Q_o = 2
9. ANALYSIS METHOD: EQUIVALENT LATERAL FORCE PROCEDURE

DC-4 FOUNDATION DESIGN CRITERIA

A. FOUNDATION DESIGN IS BASED UPON THE FOLLOWING SOIL PARAMETERS AS PROVIDED IN THE GEOTECHNICAL ENGINEERING REPORT LISTED BELOW:
1. REPORT AGENCY UES
2. REPORT # 4030.2400199
3. REPORT DATE 2025-04-17
B. NET ALLOWABLE SOIL BEARING PRESSURE
1. SPREAD FOOTINGS 3000 PSF
2. CONTINUOUS FOOTINGS 3000 PSF
C. LATERAL EARTH PRESSURE PARAMETERS
1. SOIL DENSITY 120 PCF
2. ANGLE OF INTERNAL FRICTION 30 DEGREES
3. COEFFICIENT OF FRICTION (u) 0.36
4. WIND/SEISMIC INCREASE 1/3 INCREASE
5. PASSIVE EARTH PRESSURE (Kp) 3.00
D. MODULUS OF SUB-GRADE REACTION (ks) 120 PCI
E. MINIMUM BEARING DEPTH 24 INCHES

DC-5 ANTITERRORISM (AT) CRITERIA

A. THIS FACILITY HAS BEEN DESIGNED IN ACCORDANCE WITH THE ANTITERRORISM REQUIREMENTS SET FORTH IN UFC 4-010-01, DATED 24 MAY 2024. BUILDING ANTITERRORISM STRUCTURAL DESIGN CRITERIA ARE AS FOLLOWS:
B. AT FACILITY CRITERIA
1. STANDARD 1: BUILDING STANDOFF DISTANCE > 50 FT TO PERIMETER
2. STANDARD 2: UNOBSTRUCTED SPACE 33 FT
3. STANDARD 5: PARKING BENEATH BUILDINGS OR ON ROOFTOPS N/A
4. STANDARD 6: PROGRESSIVE COLLAPSE N/A
5. STANDARD 7: STRUCTURAL ISOLATION N/A
6. STANDARD 8: BUILDING OVERHANGS AND BREEZEWAYS N/A
7. STANDARD 9: EXTERIOR MASONRY WALLS #5@32" OC MAX VERTICAL REINF OVERHEAD MOUNTED ARCH FEATURES
8. STANDARD 15: RE: DELEGATED DESIGN EQUIPMENT BRACING
9. STANDARD 19: RE: DELEGATED DESIGN

US Army Corps of Engineers®

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SOLICITATION NO.: W812PL23RA0012
CONTRACT NO.: W812PL25C0037
DESIGNED BY: A. VALENCIA
DRAWN BY: R. CARLSON
CHECKED BY: D. CLAYSON
SUBMITTED BY: P. PASZCZUK
SIZE: ANSI D

US ARMY CORPS OF ENGINEERS
LOS ANGELES DISTRICT
KORTE CONSTRUCTION
5700 OAKLAND AVE, SUITE 275
ST. LOUIS, MO 63110

CREECH AIR FORCE BASE, CLARK COUNTY, NV
DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2
494137
STRUCTURAL DESIGN CRITERIA, LEGEND, AND ABBREVIATIONS

SHEET ID
S-001

PRELIMINARY DESIGN
NOT FOR CONSTRUCTION

DP-1 95% SUBMISSION

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A

CONCRETE REINFORCING DEVELOPMENT AND LAP SPLICE TABLE

ld

DEVELOPMENT LENGTH

lst

LAP SPLICE LENGTH

ldh

HOOK DEVELOPMENT LENGTH

BAR SIZE

#3

#4

#5

#6

#7

#8

#9

#10

#11

ld (TOP BARS)

18

24

30

35

51

59

66

74

82

ld (OTHER BARS)

14

18

23

27

40

45

51

57

64

lst (TOP BARS)

24

32

39

46

67

77

86

97

107

lst (OTHER BARS)

18

24

30

35

51

59

66

74

82

ldh

12

12

12

14

17

19

21

24

27

BAR SIZE

#3

#4

#5

#6

#7

#8

#9

#10

#11

ld (TOP BARS)

17

23

28

34

49

56

63

71

78

ld (OTHER BARS)

13

17

22

26

38

43

48

54

60

lst (TOP BARS)

23

30

37

45

64

73

82

93

102

lst (OTHER BARS)

17

23

28

34

49

56

63

71

78

ldh

12

12

12

14

16

18

20

23

25

NOTES:

1. LENGTHS SHOWN ARE IN INCHES.

2. LENGTHS SHOWN ABOVE ARE FOR SINGLE REINFORCING BARS WITH MAXIMUM YIELD STRENGTH OF 60KSI.

3. LENGTHS SHOWN ASSUME CLEAR SPACING OF BARS ARE AT LEAST 2 TIMES BAR DIAMETER AND CLEAR COVER OF AT...

4. LENGTHS SHOWN ABOVE ARE FOR NORMAL WEIGHT CONCRETE. FOR LIGHT WEIGHT CONCRETE, MULTIPLY VALUES BY...

5. FOR EPOXY COATED BARS, MULTIPLY VALUES BY 1.5.

6. WHEN SPLICING BARS OF DIFFERENT SIZES, USE SPLICE LENGTH FOR LARGER BAR.

7. SPLICES (LAPS) OF REINFORCING BARS MUST BE CLASS 'B' TENSION LAPS PER ACI 318, UNLESS NOTED OTHERWISE.

8. TOP BARS ARE HORIZONTAL REINFORCING BARS WITH 12 INCHES OR MORE OF FRESH CONCRETE IS PLACED BELOW.

9. OTHER BARS ARE ANY REINFORCING BARS THAT DO NOT MEET QUALIFICATION FOR TOP BARS.

MASONRY STRENGTH TABLE

ELEMENT

CONCRETE MASONRY

SPECIFIED COMPRESSIVE STRENGTH

f_m = 2000 PSI

GROUT FOR CONCRETE MASONRY

f_g ≥ f_m (2000 PSI MINIMUM)

NOTES:

1. PROVIDE MEDIUM WEIGHT HOLLOW CONCRETE MASONRY UNITS FOR GENERAL USE UNLESS OTHERWISE NOTED.

2. MORTAR FOR CONCRETE MASONRY MUST BE TYPE S AT EXTERIOR WALLS AND TYPE N AT INTERIOR WALLS.

MASONRY REINFORCING SPLICE TABLE

BAR SIZE

#3

8" CMU

12

10" CMU

12

12" CMU

12

8" CMU

13

10" CMU

13

12" CMU

13

#4

13

12

12

22

22

22

#5

19

16

13

35

35

35

#6

37

29

24

54

54

54

#7

-

40

33

-

63

63

#8

-

-

50

-

-

72

NOTES:

1. LAP SPLICE LENGTHS ARE IN INCHES.

2. LAP SPLICES IN REINFORCED MASONRY MUST HAVE MINIMUM LENGTHS AS DEFINED ABOVE UNLESS NOTED OTHERWISE

3. TABULATED VAULES ARE CALCUATED IN ACCORDANCE WITH TMS 402/602-16 CHAPTER 6.

4. SPLICE AND DEVELOPMENT LENGTHS ARE THE SAME VALUE FOR HORIZONTAL AND VERTICAL BARS

5. SINGLE REINFORCING IS A SINGLE BAR CENTERED IN CMU BLOCK CELL. DOUBLE REINFORCING IS TWO BARS IN A CMU BLOCK CELL WITH 2 INCH MINIMUM CLEAR COVER FROM OUTSIDE FACE OF BLOCK.

6. TABULATED VALUES BASED ON UNCOATED REINFORCMENT WITH A YEILD STRENGTH, F_y =60 KSI

7. TABULATED VALUES BASED ON MASONRY COMPRESSIVE STRENGTH, f_m =2000 PSI

US Army Corps of Engineers

of Engineers®

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CHECKED BY:
D. CLAYSON

SUBMITTED BY:

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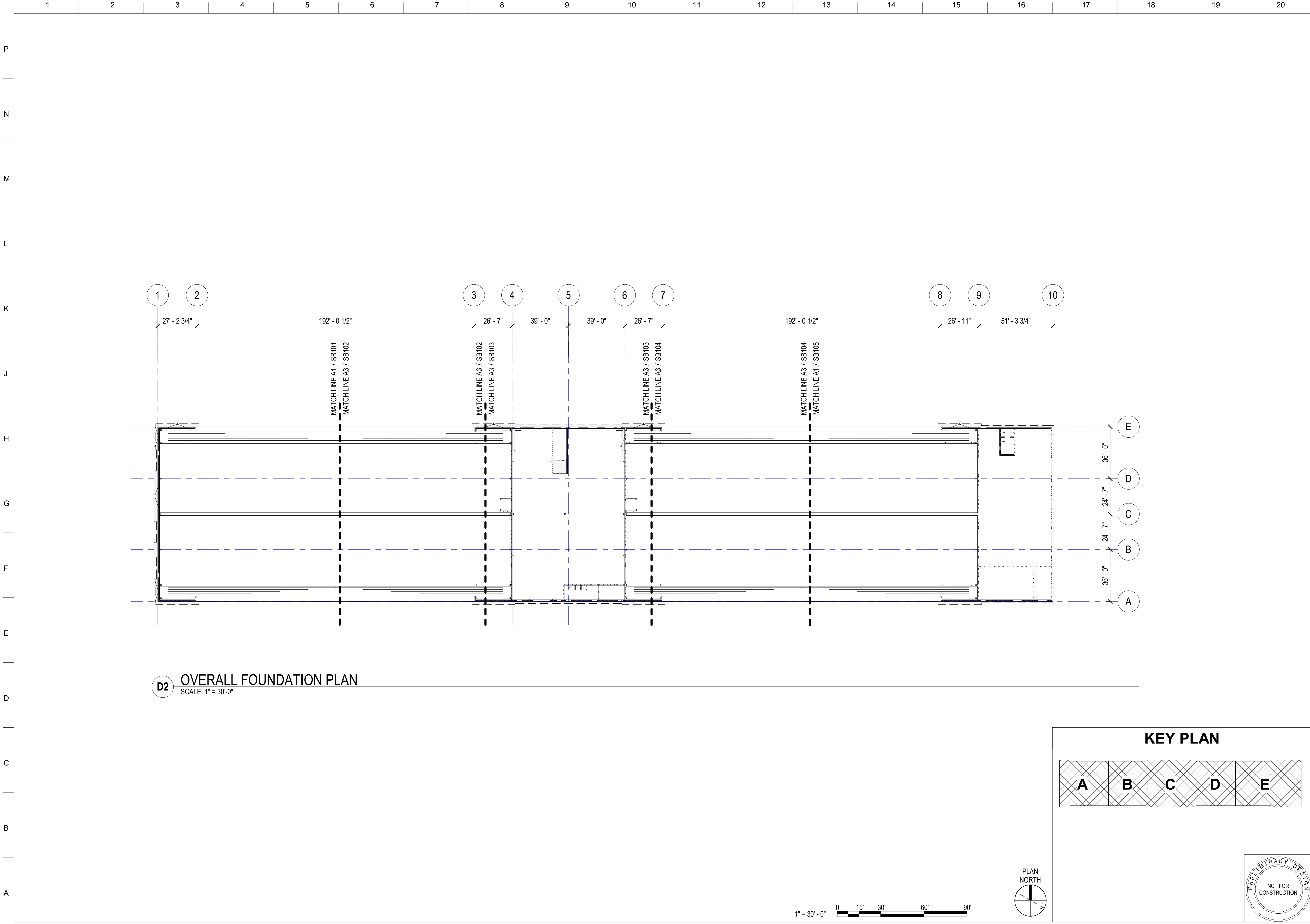
CREECH AIR FORCE BASE, CLARK COUNTY, NV
DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2
494137

GENERAL STRUCTURAL NOTES

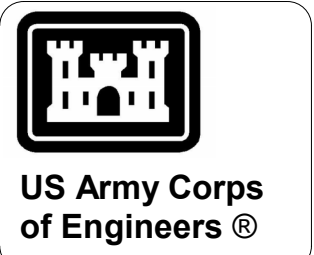
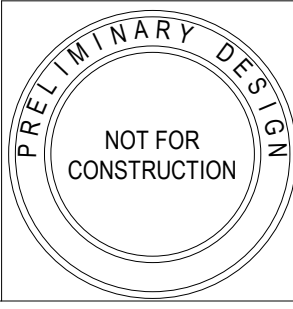
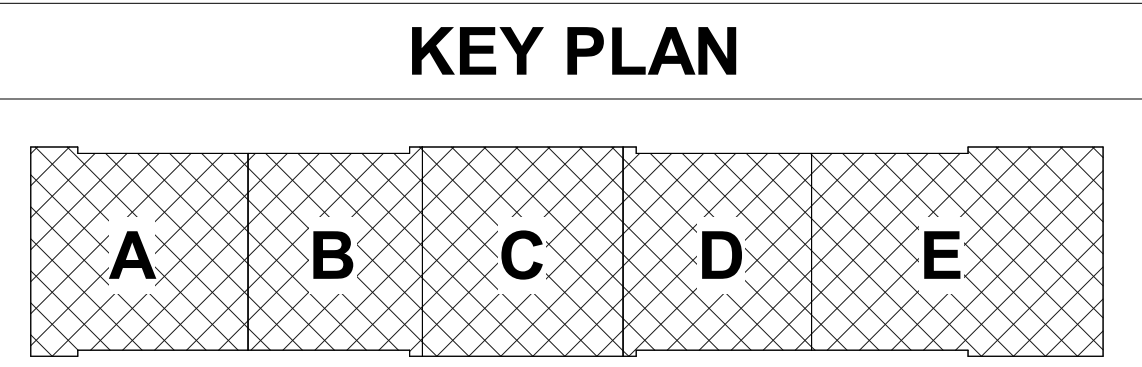
PRELIMINARY DESIGN
NOT FOR CONSTRUCTION

SHEET ID
S-004

DP-1 95% SUBMISSION



D2 OVERALL FOUNDATION PLAN
SCALE: 1" = 30'-0"

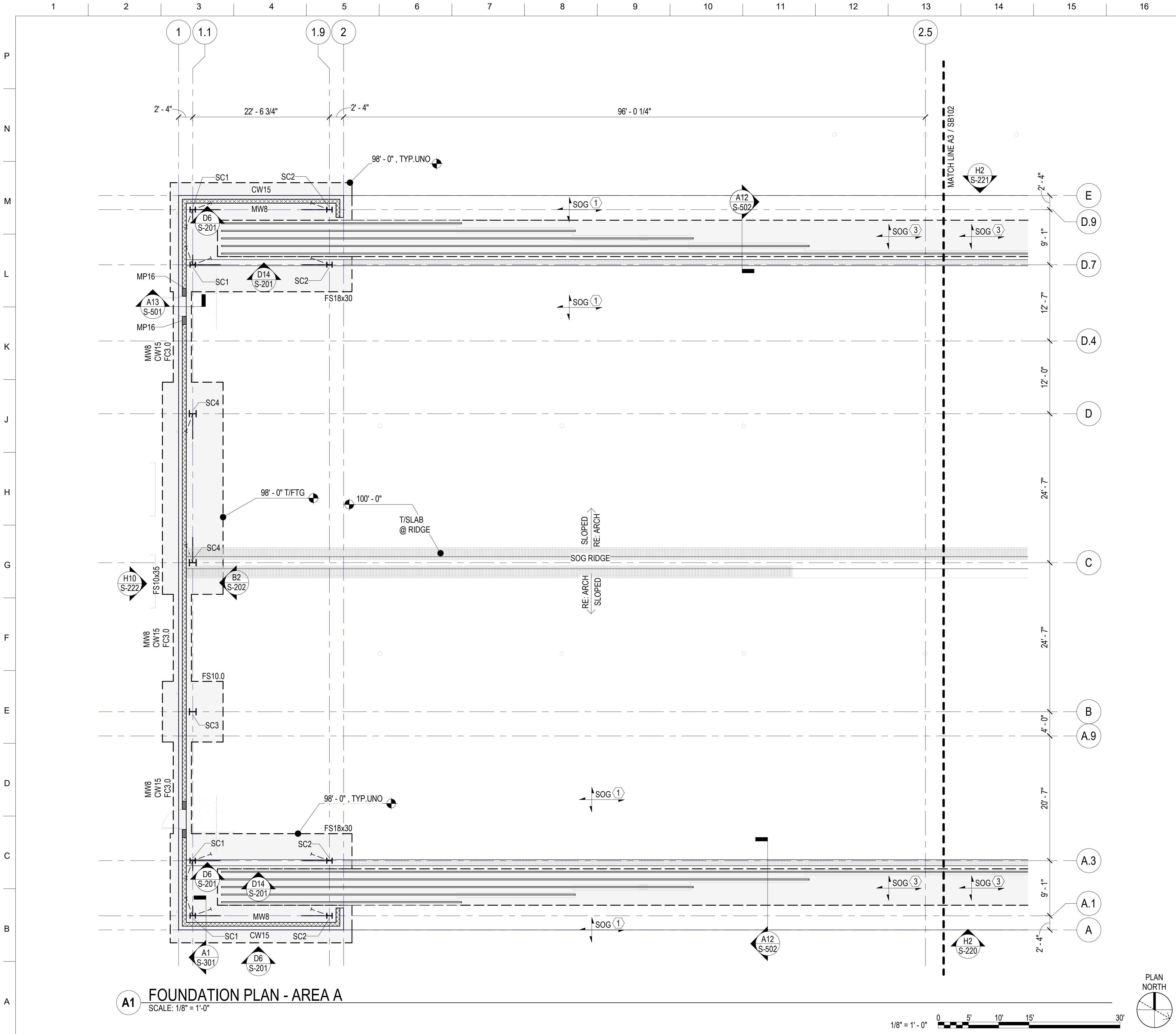


MARK				DESCRIPTION	DATE

DESIGNED BY: A. VALENCIA DRAWN BY: R. CARLSON CHECKED BY: D. CLAYSON SUBMITTED BY: P. PASZCZUK SIZE: ANSI D	ISSUE DATE: NOVEMBER 13, 2025
	SOLICITATION NO.: W812PL23RA0012
	CONTRACT NO.: W812PL25C0037
US ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT	
KORTE CONSTRUCTION 5700 OAKLAND AVE, SUITE 275 ST. LOUIS, MO 63110	

CREECH AIR FORCE BASE, CLARK COUNTY, NV
DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2
494.37
OVERALL FOUNDATION PLAN

SHEET ID
SB100



GENERAL NOTES

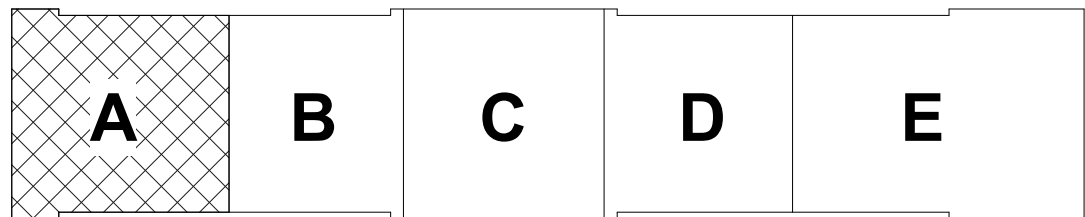
1. SEE SHEETS S-001 TO S-005 FOR GENERAL NOTES AND DESIGN CRITERIA.
2. SEE ARCHITECTURAL, CIVIL, AND LANDSCAPE DRAWINGS FOR EXTERIOR CONCRETE WORK AT DOORS, SIDEWALKS, ETC.
3. TYPICAL TOP OF SLAB ELEVATION = RE: CIVIL AND ARCH. THIS IS THE DATUM 100'-0". ALL ELEVATIONS ARE REFERENCED FROM THIS ELEVATION.
4. FOUNDATIONS ARE SPREAD, STRIP, MAT FOOTINGS. ALL FOOTINGS TO BE CENTERED BELOW WALLS AND COLUMNS (OR COLUMN GROUP) UNLESS NOTED OTHERWISE.
5. ALL TOP OF FOOTING ELEVATIONS = 98'-0" UNLESS NOTED OTHERWISE.
6. ALL EXTERIOR FOOTINGS MUST BEAR AT OR BELOW MINIMUM BEARING DEPTH AS SPECIFIED IN DESIGN CRITERIA SECTION. PROVIDE ADDITIONAL FOOTING STEPS AS REQUIRED. CONTRACTOR SHALL COORDINATE ALL FOOTING STEP LOCATIONS.
7. RE: PLAN FOR SLAB ON GRADE THICKNESS AND REINFORCING.
8. FOR DIMENSIONS NOT SHOWN ON THESE SHEETS, COORDINATE WALL OPENINGS, ELEVATIONS, SECTIONS, AND DETAILS WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS.
9. FOR UTILITIES PASSING BELOW FOOTINGS, SEE L17/S-501. CONTRACTOR MUST COORDINATE ALL UTILITY AND FOOTING STEP LOCATIONS. SEE L13/S-501 FOR TYPICAL FOOTING STEP DETAIL AND D9/S-501 FOR UTILITY PENETRATION THROUGH FOUNDATION WALLS.
10. ALL FOOTINGS AT THE END OF A WALL THAT DOES NOT END IN A CORNER CONDITION SHALL EXTEND 1'-0" PAST WALL END.
11. CMU WALLS ARE MW8 UNLESS NOTED OTHERWISE.
12. CONCRETE STEM WALLS ARE CW8 UNDER MW8 CMU AND CW15 UNDER MW8 WALLS WITH CMU VENEER UNLESS NOTED OTHERWISE.
13. RE: ARCH DRAWINGS FOR SLAB SLOPES AND DRAINS, ETC.

KEYNOTES

- ① 12 1/2" THICK UNREINFORCED SLAB ON GRADE ON 15 MIL VAPOR RETARDER OVER 6" MIN OF CRUSHED STONE AGGREGATE AND 12" MIN COMPACTED NON-EXPANSIVE FILL (SEE GEOTECH REPORT). SLAB ON GROUND JOINTS MUST BE LOCATED NO GREATER THAN 20 FT ON CENTER EACH DIRECTION, RE: E6/S-502 AND E10/S-502.)
- ② 6" THICK REINFORCED SLAB ON GRADE W/ # 4 @ 18" OC EACH WAY ON 15 MIL VAPOR RETARDER OVER 6" MIN OF CRUSHED STONE AGGREGATE AND 12" MIN COMPACTED NON-EXPANSIVE FILL (SEE GEOTECH REPORT). SLAB ON GROUND JOINTS MUST BE LOCATED NO GREATER THAN 15 FT ON CENTER EACH DIRECTION, RE: E6/S-502 AND E10/S-502.)
- ③ 8" THICK UNREINFORCED CONC. SLAB OVER CONC. MAT FOOTING AT HANGAR DOOR SUPPORT RAILS. RE: A12/S-502
- ④ RE: H9/S-501 FOR SLAB REINFORCING AT OPENINGS AND RE-ENTRANT CORNERS, TYP

TRENCH DRAIN COORDINATION
AND DETAILS IN PROGRESS

KEY PLAN

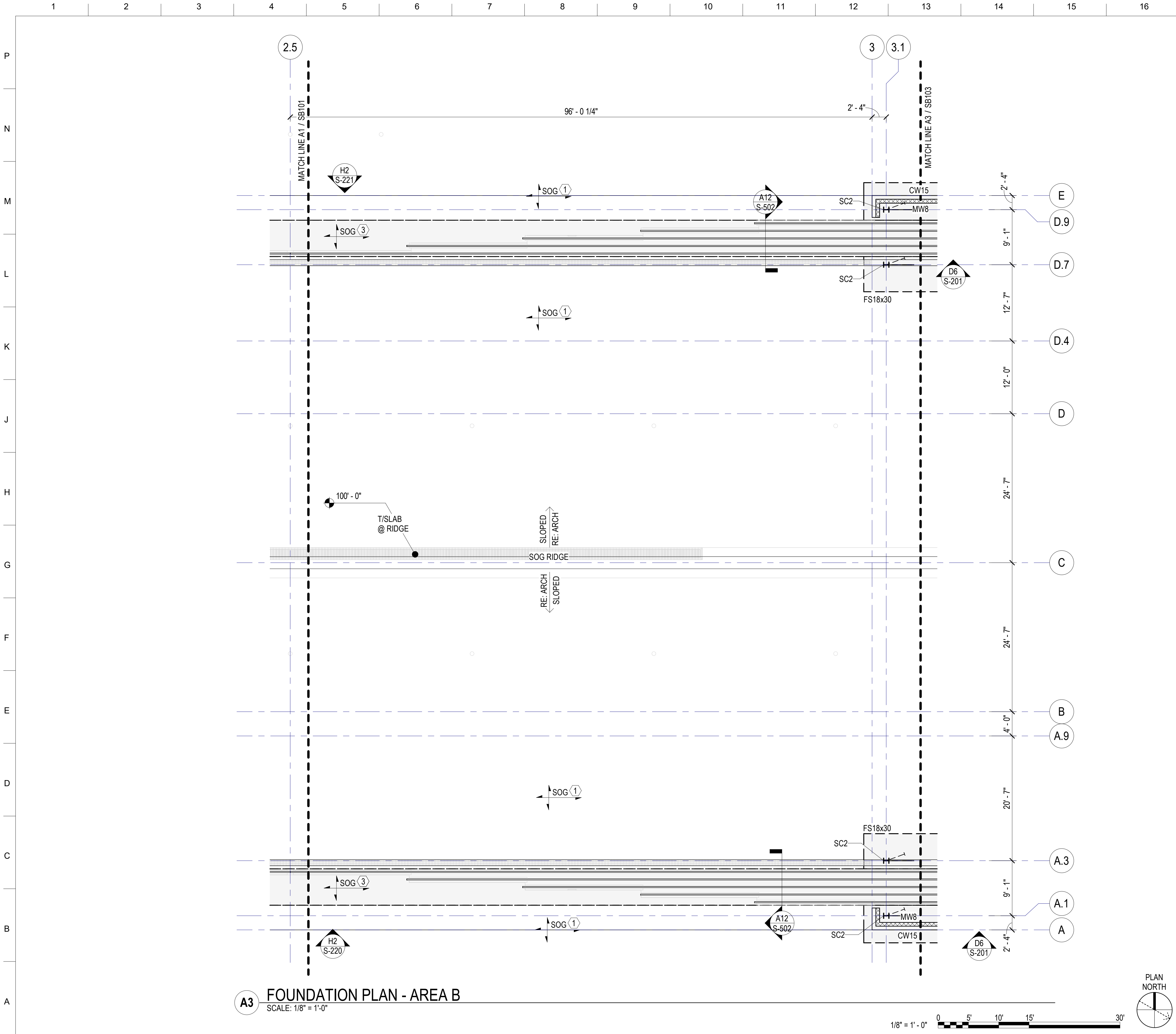


DATE		DESCRIPTION	MARK

ISSUE DATE: NOVEMBER 13, 2025	DESIGNED BY: A. VALENCIA	US ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT
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	SUBMITTED BY: P. PASZCZUK	
	SIZE: ANSI D	

CREECH AIR FORCE BASE, CLARK COUNTY, NV DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2 484.37	FOUNDATION PLAN - AREA A
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SHEET ID SB101



A3 FOUNDATION PLAN - AREA B
SCALE: 1/8" = 1'-0"

GENERAL NOTES

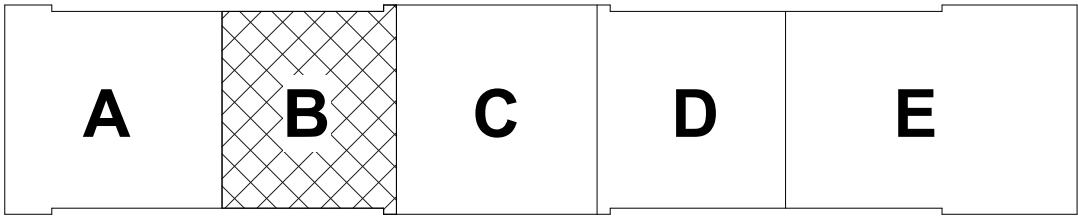
1. SEE SHEETS S-001 TO S-005 FOR GENERAL NOTES AND DESIGN CRITERIA.
2. SEE ARCHITECTURAL, CIVIL, AND LANDSCAPE DRAWINGS FOR EXTERIOR CONCRETE WORK AT DOORS, SIDEWALKS, ETC.
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KEYNOTES

- ① 12 1/2" THICK UNREINFORCED SLAB ON GRADE ON 15 MIL VAPOR RETARDER OVER 6" MIN OF CRUSHED STONE AGGREGATE AND 12" MIN COMPACTED NON-EXPANSIVE FILL (SEE GEOTECH REPORT). SLAB ON GROUND JOINTS MUST BE LOCATED NO GREATER THAN 20 FT ON CENTER EACH DIRECTION, RE: E6/S-502 AND E10/S-502.)
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- ③ 8" THICK UNREINFORCED CONC. SLAB OVER CONC. MAT FOOTING AT HANGAR DOOR SUPPORT RAILS. RE: A12/S-502
- ④ RE: H9/S-501 FOR SLAB REINFORCING AT OPENINGS AND RE-ENTRANT CORNERS, TYP

**TRENCH DRAIN COORDINATION
AND DETAILS IN PROGRESS**

KEY PLAN

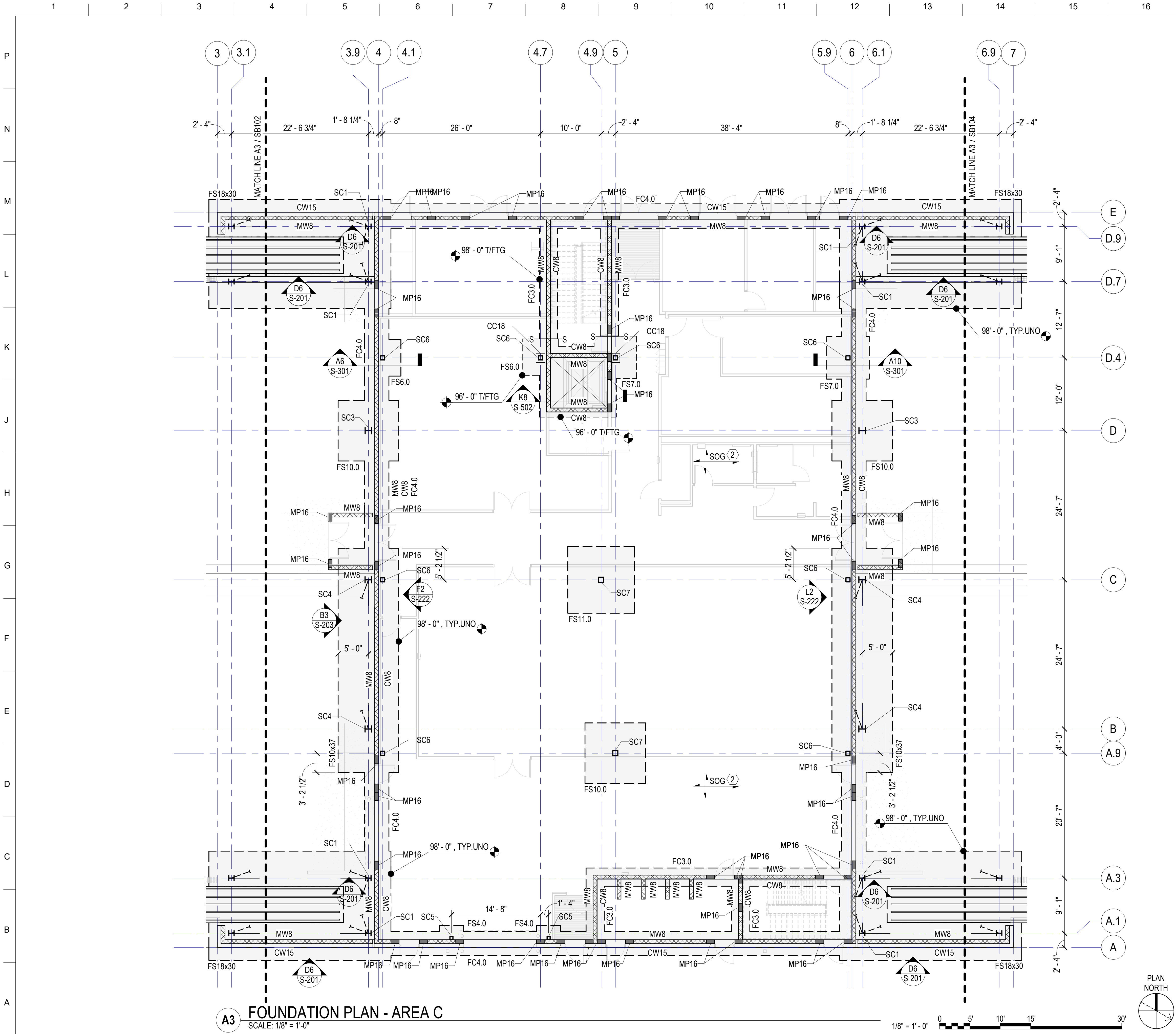


DATE		DESCRIPTION	MARK

ISSUE DATE: NOVEMBER 13, 2025	DESIGNED BY: A. VALENCIA	US ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT
SOLICITATION NO.: W812PL23RA0012	DRAWN BY: R. CARLSON	
CONTRACT NO.: W812PL25C0037	CHECKED BY: D. CLAYSON	
	SUBMITTED BY: P. PASZCZUK	
	SIZE: ANSI D	

CREECH AIR FORCE BASE, CLARK COUNTY, NV DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2 494137	FOUNDATION PLAN - AREA B
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SHEET ID SB102



GENERAL NOTES

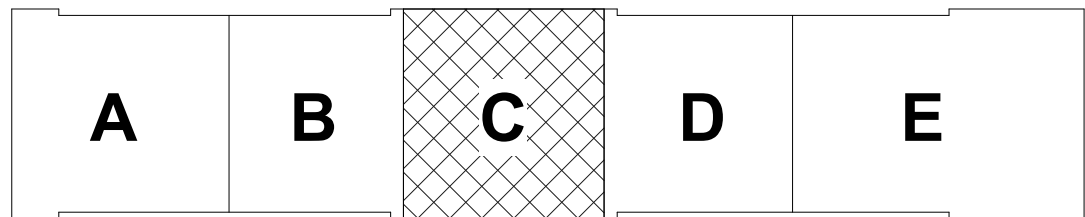
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**TRENCH DRAIN COORDINATION
AND DETAILS IN PROGRESS**

KEY PLAN

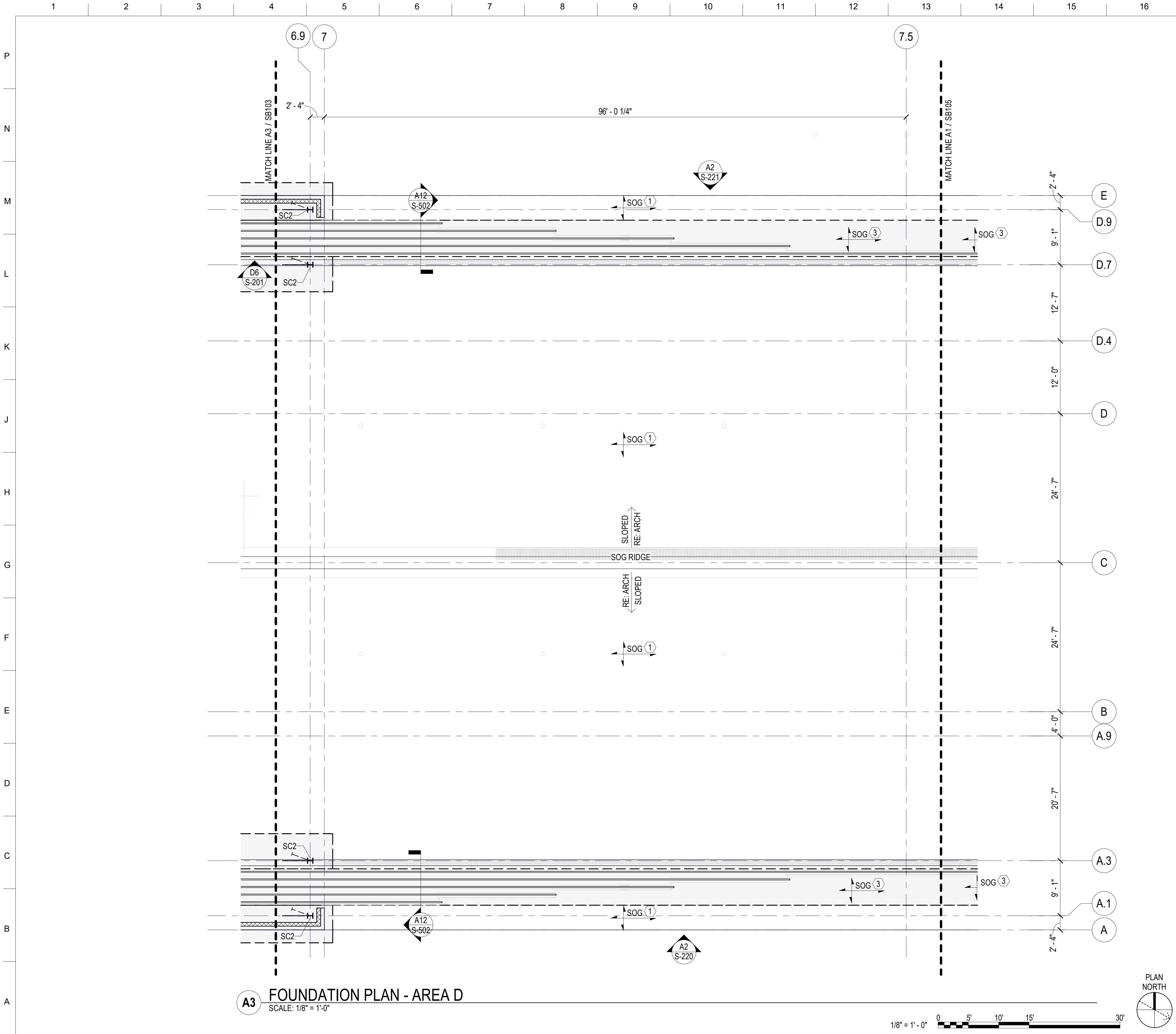


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SOLICITATION NO.: W812PL23RA0012	DRAWN BY: R. CARLSON	
CONTRACT NO.: W812PL25C0037	CHECKED BY: D. CLAYSON	KORTE CONSTRUCTION 5700 OAKLAND AVE, SUITE 275 ST. LOUIS, MO 63110
	SUBMITTED BY: P. PASZCZUK	
	SIZE: ANSI D	

CREECH AIR FORCE BASE, CLARK COUNTY, NV DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2 494.37	FOUNDATION PLAN - AREA C
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SHEET ID SB103



GENERAL NOTES

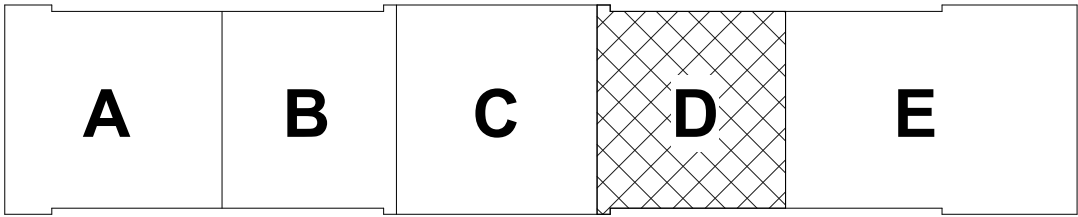
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TRENCH DRAIN COORDINATION
AND DETAILS IN PROGRESS

KEY PLAN

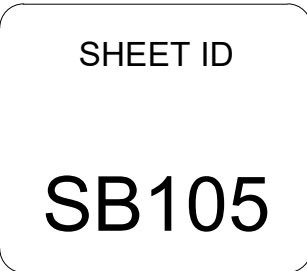
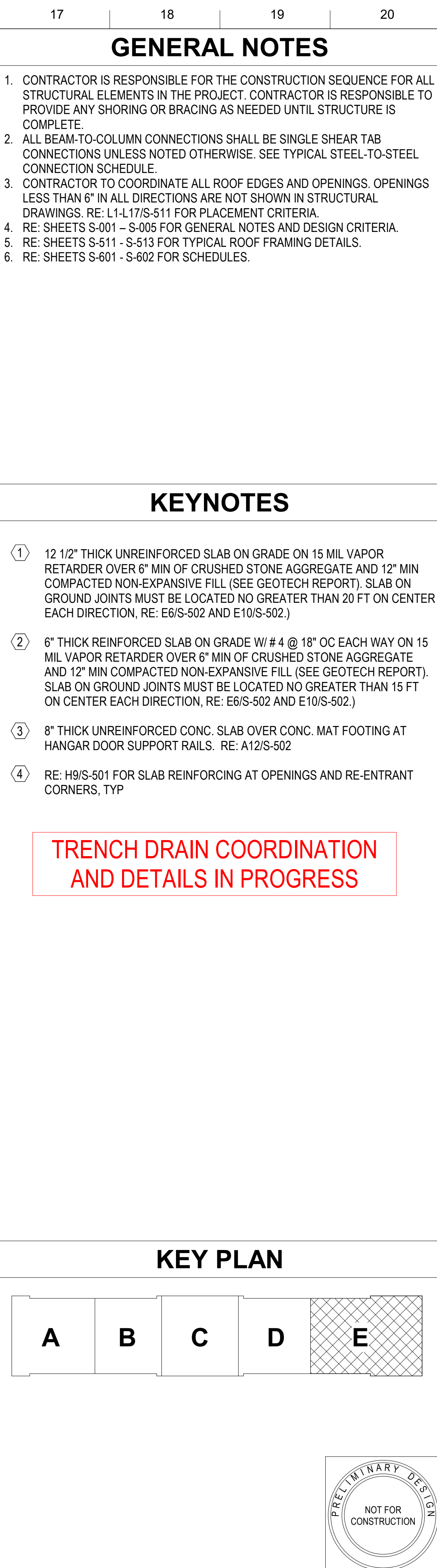


DATE		DESCRIPTION	MARK

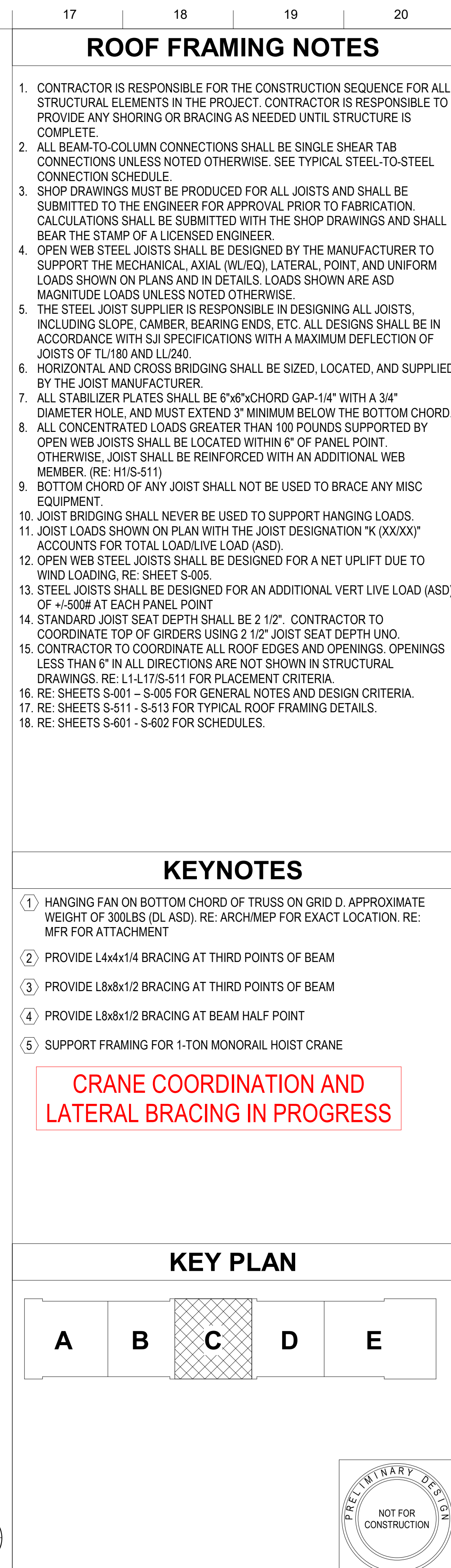
ISSUE DATE: NOVEMBER 13, 2025	DESIGNED BY: A. VALENIA	US ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT
SOLICITATION NO.: W812PL23RA0012	DRAWN BY: R. CARLSON	KORTE CONSTRUCTION 5700 OAKLAND AVE, SUITE 275 ST. LOUIS, MO 63110
CONTRACT NO.: W812PL25C0037	CHECKED BY: D. CLAYSON	
	SUBMITTED BY: P. PASZCZUK	
	SIZE: ANSI D	

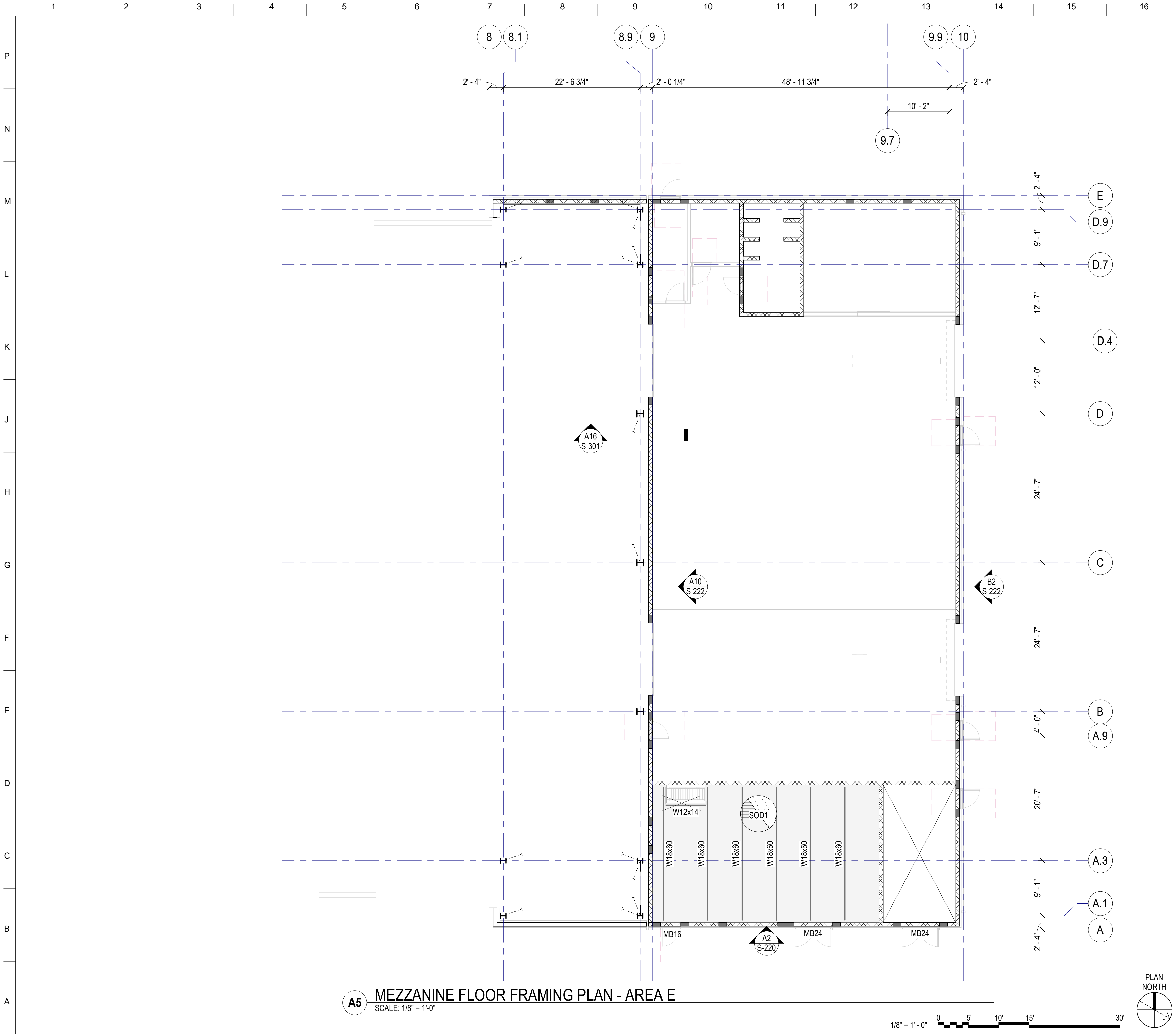
CREECH AIR FORCE BASE, CLARK COUNTY, NV DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2 494.37	FOUNDATION PLAN - AREA D
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SHEET ID SB104



DP-1 95% SUBMISSION

[illegible]



ROOF FRAMING NOTES

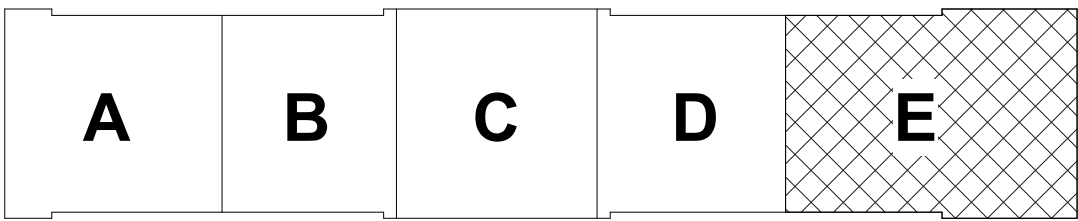
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2. ALL BEAM-TO-COLUMN CONNECTIONS SHALL BE SINGLE SHEAR TAB CONNECTIONS UNLESS NOTED OTHERWISE. SEE TYPICAL STEEL-TO-STEEL CONNECTION SCHEDULE.
3. SHOP DRAWINGS MUST BE PRODUCED FOR ALL JOISTS AND SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO FABRICATION. CALCULATIONS SHALL BE SUBMITTED WITH THE SHOP DRAWINGS AND SHALL BEAR THE STAMP OF A LICENSED ENGINEER.
4. OPEN WEB STEEL JOISTS SHALL BE DESIGNED BY THE MANUFACTURER TO SUPPORT THE MECHANICAL, AXIAL (WL/EQ), LATERAL, POINT, AND UNIFORM LOADS SHOWN ON PLANS AND IN DETAILS. LOADS SHOWN ARE ASD MAGNITUDE LOADS UNLESS NOTED OTHERWISE.
5. THE STEEL JOIST SUPPLIER IS RESPONSIBLE IN DESIGNING ALL JOISTS, INCLUDING SLOPE, CAMBER, BEARING ENDS, ETC. ALL DESIGNS SHALL BE IN ACCORDANCE WITH SJI SPECIFICATIONS WITH A MAXIMUM DEFLECTION OF JOISTS OF TL/180 AND LL/240.
6. HORIZONTAL AND CROSS BRIDGING SHALL BE SIZED, LOCATED, AND SUPPLIED BY THE JOIST MANUFACTURER.
7. ALL STABILIZER PLATES SHALL BE 6"x6"xCHORD GAP-1/4" WITH A 3/4" DIAMETER HOLE, AND MUST EXTEND 3" MINIMUM BELOW THE BOTTOM CHORD.
8. ALL CONCENTRATED LOADS GREATER THAN 100 POUNDS SUPPORTED BY OPEN WEB JOISTS SHALL BE LOCATED WITHIN 6" OF PANEL POINT. OTHERWISE, JOIST SHALL BE REINFORCED WITH AN ADDITIONAL WEB MEMBER. (RE: H1/S-511)
9. BOTTOM CHORD OF ANY JOIST SHALL NOT BE USED TO BRACE ANY MISC EQUIPMENT.
10. JOIST BRIDGING SHALL NEVER BE USED TO SUPPORT HANGING LOADS.
11. JOIST LOADS SHOWN ON PLAN WITH THE JOIST DESIGNATION "K (XX/XX)" ACCOUNTS FOR TOTAL LOAD/LIVE LOAD (ASD).
12. OPEN WEB STEEL JOISTS SHALL BE DESIGNED FOR A NET UPLIFT DUE TO WIND LOADING, RE: SHEET S-005.
13. STEEL JOISTS SHALL BE DESIGNED FOR AN ADDITIONAL VERT LIVE LOAD (ASD) OF +/-500# AT EACH PANEL POINT
14. STANDARD JOIST SEAT DEPTH SHALL BE 2 1/2". CONTRACTOR TO COORDINATE TOP OF GIRDERS USING 2 1/2" JOIST SEAT DEPTH UNO.
15. CONTRACTOR TO COORDINATE ALL ROOF EDGES AND OPENINGS. OPENINGS LESS THAN 6" IN ALL DIRECTIONS ARE NOT SHOWN IN STRUCTURAL DRAWINGS. RE: L1-L17/S-511 FOR PLACEMENT CRITERIA.
16. RE: SHEETS S-001 - S-005 FOR GENERAL NOTES AND DESIGN CRITERIA.
17. RE: SHEETS S-511 - S-513 FOR TYPICAL ROOF FRAMING DETAILS.
18. RE: SHEETS S-601 - S-602 FOR SCHEDULES.

KEYNOTES

- ① HANGING FAN ON BOTTOM CHORD OF TRUSS ON GRID D. APPROXIMATE WEIGHT OF 300LBS (DL ASD). RE: ARCH/MEP FOR EXACT LOCATION. RE: MFR FOR ATTACHMENT
- ② PROVIDE L4x4x1/4 BRACING AT THIRD POINTS OF BEAM
- ③ PROVIDE L8x8x1/2 BRACING AT THIRD POINTS OF BEAM
- ④ PROVIDE L8x8x1/2 BRACING AT BEAM HALF POINT
- ⑤ SUPPORT FRAMING FOR 1-TON MONORAIL HOIST CRANE

CRANE COORDINATION AND
LATERAL BRACING IN PROGRESS

KEY PLAN



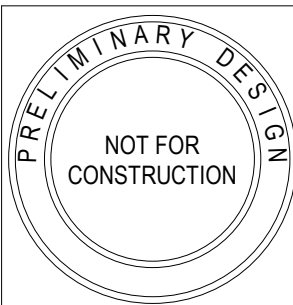
US Army Corps
of Engineers ®

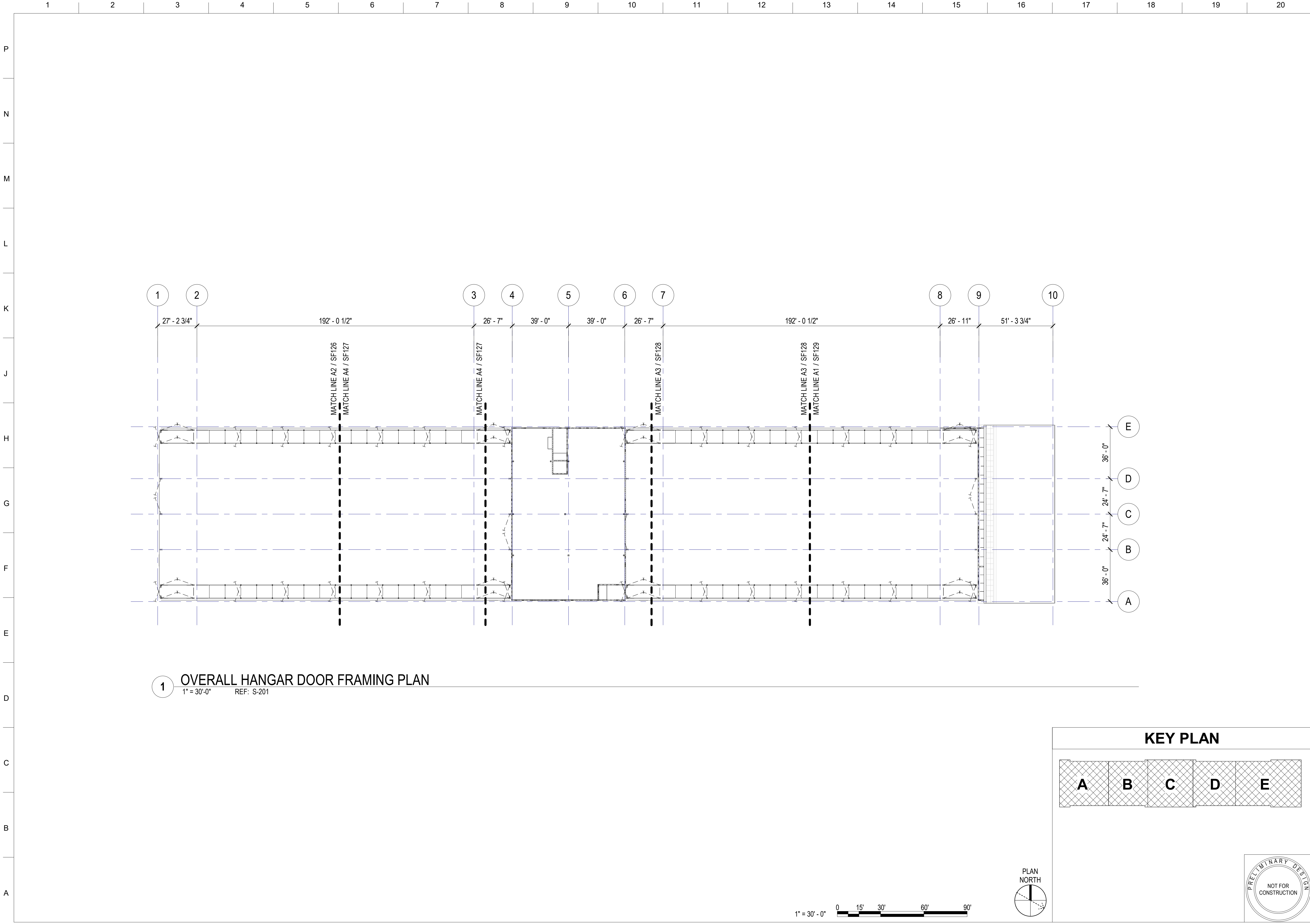
MARK				DESCRIPTION	DATE

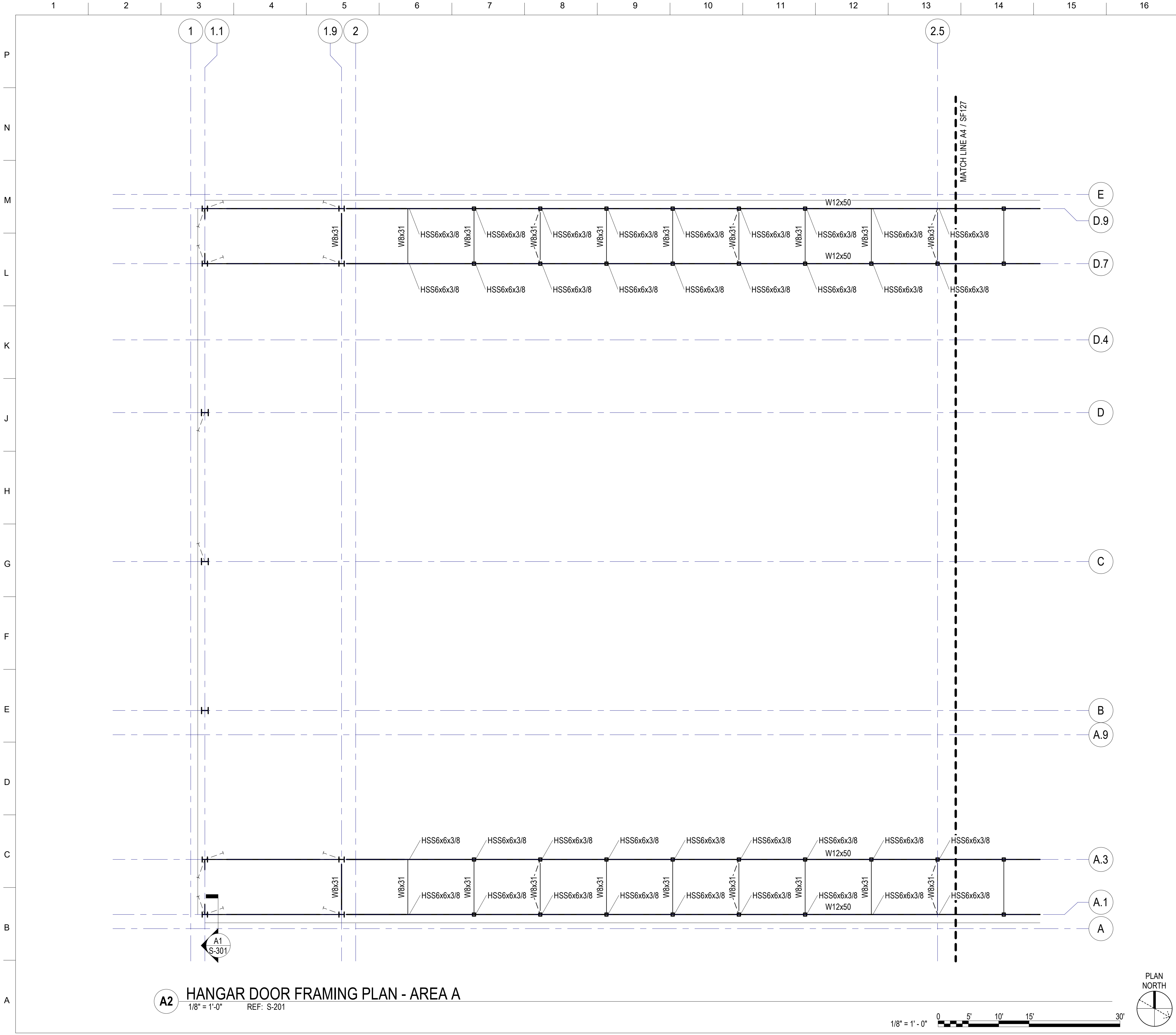
DESIGNED BY: A. VALENCIA	ISSUE DATE: NOVEMBER 13, 2025
DRAWN BY: R. CARLSON	SOLICITATION NO.: WB12PL23RA0012
CHECKED BY: D. CLAYSON	CONTRACT NO.: WB12PL25C0037
SUBMITTED BY: P. PASZCZUK	SIZE: ANSI D
US ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT	
KORTE CONSTRUCTION 5700 OAKLAND AVE, SUITE 275 ST. LOUIS, MO 63110	

CREECH AIR FORCE BASE, CLARK COUNTY, NV DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2 494137 MEZZANINE FLOOR FRAMING PLAN - AREA E

SHEET ID SF122







A2 HANGAR DOOR FRAMING PLAN - AREA A
1/8" = 1'-0" REF: S-201

FRAMING NOTES

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6. RE: SHEETS S-601 - S-602 FOR SCHEDULES.

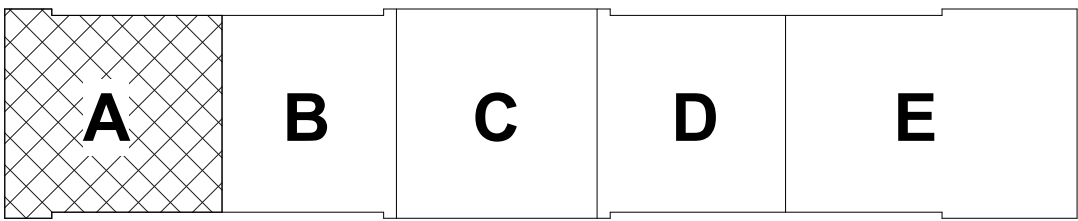


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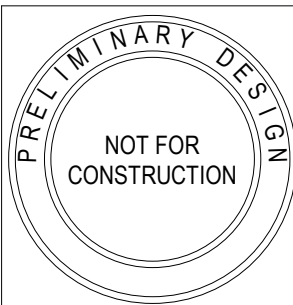
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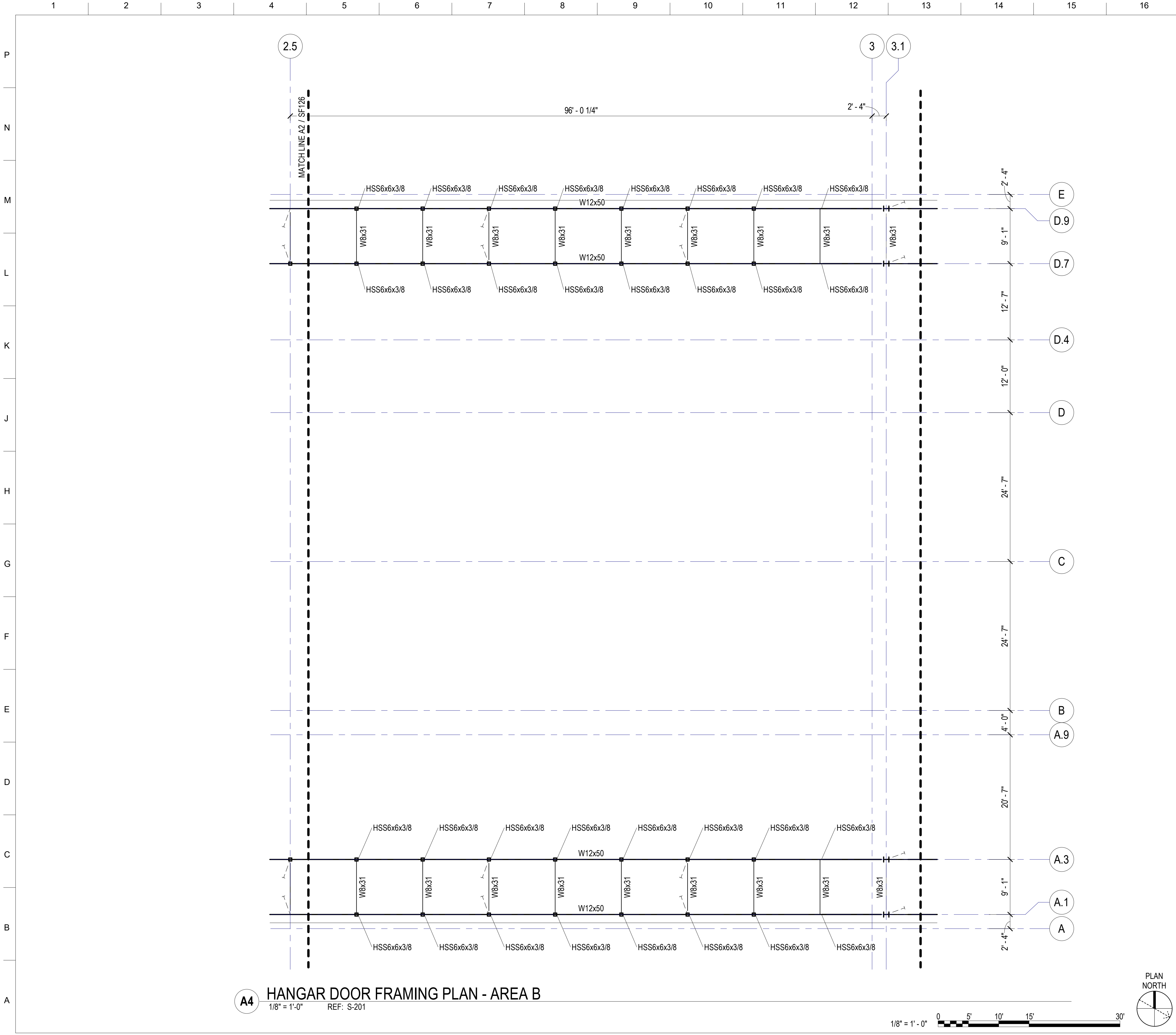
KEY PLAN



CREECH AIR FORCE BASE, CLARK COUNTY, NV
DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2
494.37
HANGAR DOOR FRAMING PLAN - AREA A

SHEET ID
SF126

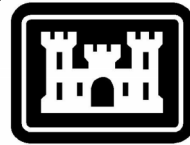




A4 HANGAR DOOR FRAMING PLAN - AREA B
1/8" = 1'-0" REF: S-201

GENERAL NOTES

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US Army Corps
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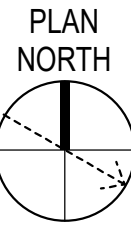
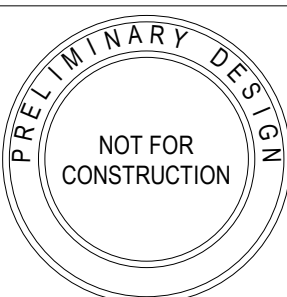
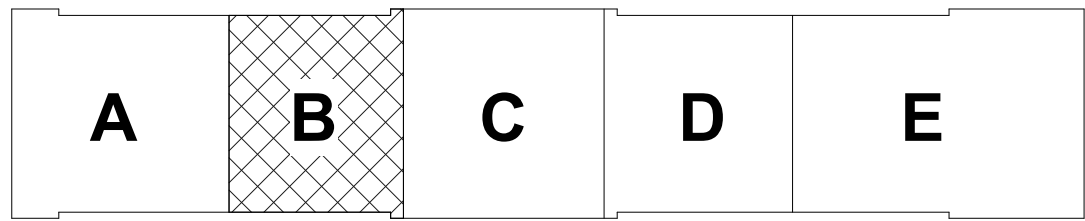
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US ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT	KORTE CONSTRUCTION 5700 OAKLAND AVE, SUITE 275 ST. LOUIS, MO 63110
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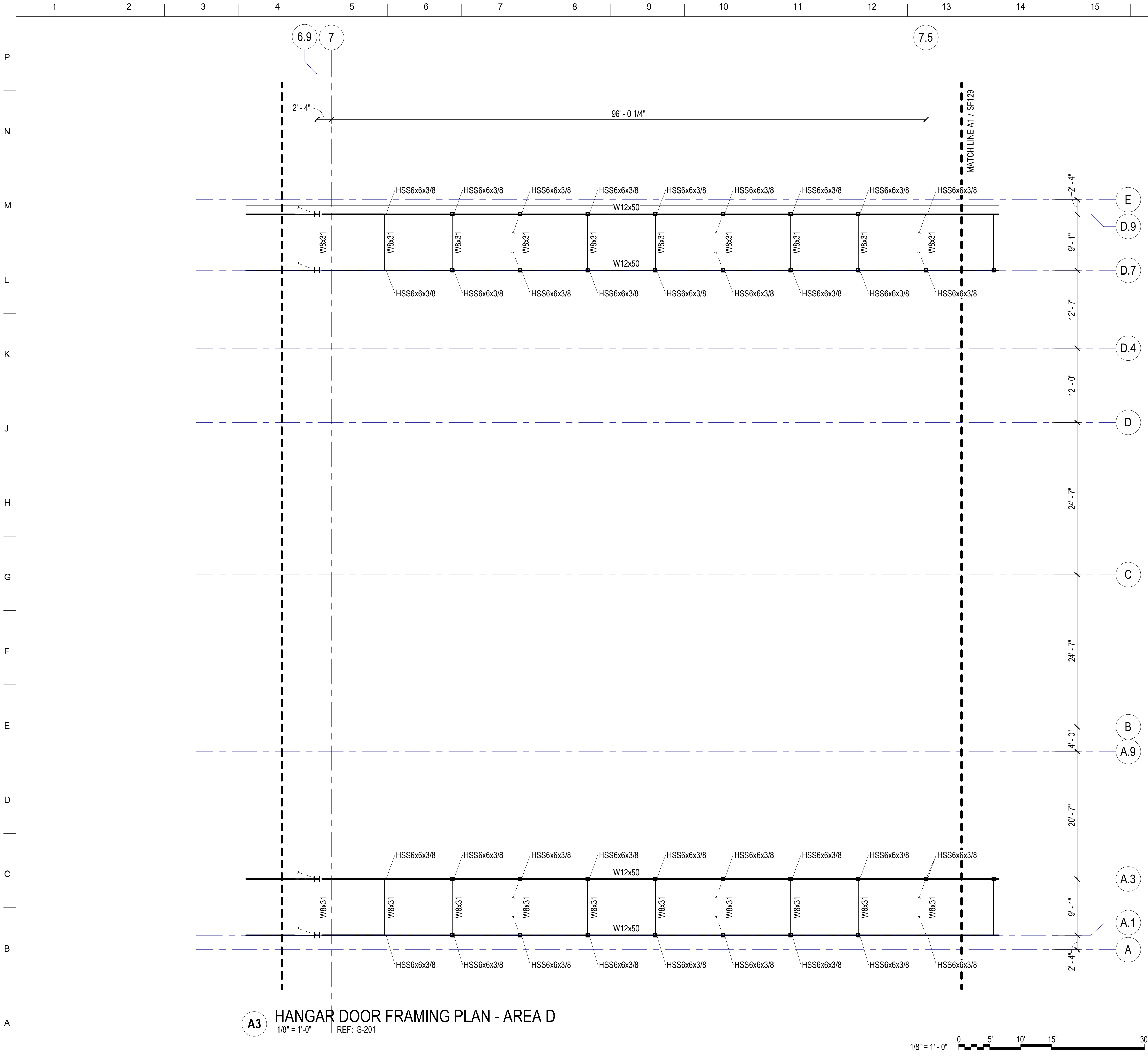
CREECH AIR FORCE BASE, CLARK COUNTY, NV DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2 494137 HANGAR DOOR FRAMING PLAN - AREA B

SHEET ID SF127

KEY PLAN

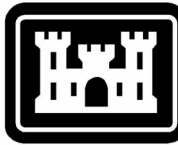


1/8" = 1'-0" 0 5' 10' 15' 30'



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7. RE: PLAN FOR SLAB ON GRADE THICKNESS AND REINFORCING.
8. FOR DIMENSIONS NOT SHOWN ON THESE SHEETS, COORDINATE WALL OPENINGS, ELEVATIONS, SECTIONS, AND DETAILS WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS.
9. FOR UTILITIES PASSING BELOW FOOTINGS, SEE L17/S-501. CONTRACTOR MUST COORDINATE ALL UTILITY AND FOOTING STEP LOCATIONS. SEE L13/S-501 FOR TYPICAL FOOTING STEP DETAIL AND D9/S-501 FOR UTILITY PENETRATION THROUGH FOUNDATION WALLS.
10. ALL FOOTINGS AT THE END OF A WALL THAT DOES NOT END IN A CORNER CONDITION SHALL EXTEND 1'-0" PAST WALL END.
11. CMU WALLS ARE MW8 UNLESS NOTED OTHERWISE.
12. CONCRETE STEM WALLS ARE CW8 UNDER MW8 CMU AND CW15 UNDER MW8 WALLS WITH CMU VENEER UNLESS NOTED OTHERWISE.
13. RE: ARCH DRAWINGS FOR SLAB SLOPES AND DRAINS, ETC.

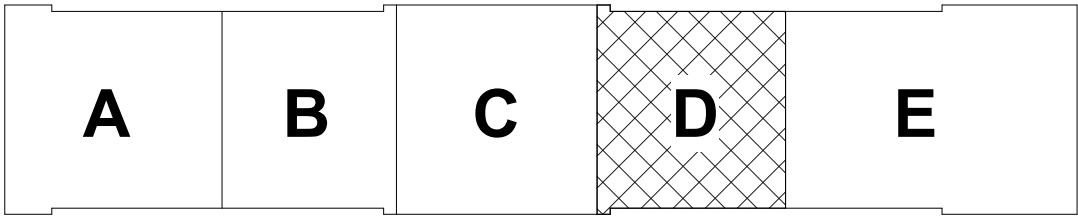


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of Engineers®

MARK	DESCRIPTION	DATE

DESIGNED BY: A. VALENCIA	ISSUE DATE: NOVEMBER 13, 2025
DRAWN BY: R. CARLSON	SOLICITATION NO.: W812PL23RA0012
CHECKED BY: D. CLAYSON	CONTRACT NO.: W812PL25C0037
SUBMITTED BY: P. PASZCZUK	
SIZE: ANSI D	
US ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT	
KORTE CONSTRUCTION 5700 OAKLAND AVE. SUITE 275 ST. LOUIS, MO 63110	

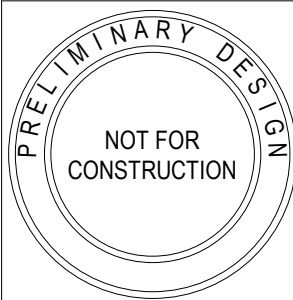
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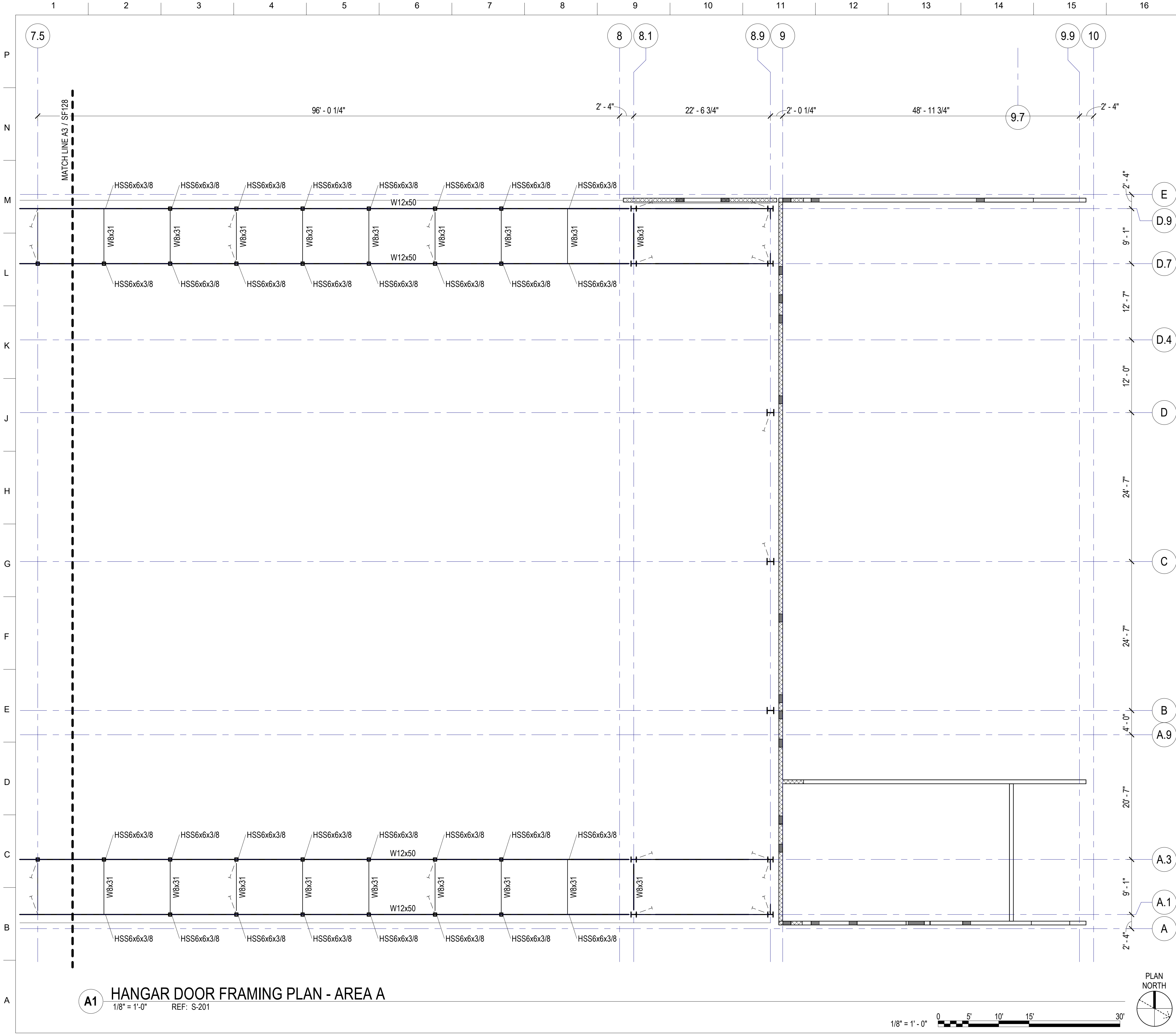


CREECH AIR FORCE BASE, CLARK COUNTY, NV
DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2
494.37
HANGAR DOOR FRAMING PLAN - AREA D

SHEET ID

SF128





GENERAL NOTES

1. CONTRACTOR IS RESPONSIBLE FOR THE CONSTRUCTION SEQUENCE FOR ALL STRUCTURAL ELEMENTS IN THE PROJECT. CONTRACTOR IS RESPONSIBLE TO PROVIDE ANY SHORING OR BRACING AS NEEDED UNTIL STRUCTURE IS COMPLETE.
2. ALL BEAM-TO-COLUMN CONNECTIONS SHALL BE SINGLE SHEAR TAB CONNECTIONS UNLESS NOTED OTHERWISE. SEE TYPICAL STEEL-TO-STEEL CONNECTION SCHEDULE.
3. CONTRACTOR TO COORDINATE ALL ROOF EDGES AND OPENINGS. OPENINGS LESS THAN 6" IN ALL DIRECTIONS ARE NOT SHOWN IN STRUCTURAL DRAWINGS. RE: L1-L17/S-511 FOR PLACEMENT CRITERIA.
4. RE: SHEETS S-001 - S-005 FOR GENERAL NOTES AND DESIGN CRITERIA.
5. RE: SHEETS S-511 - S-513 FOR TYPICAL ROOF FRAMING DETAILS.
6. RE: SHEETS S-601 - S-602 FOR SCHEDULES.

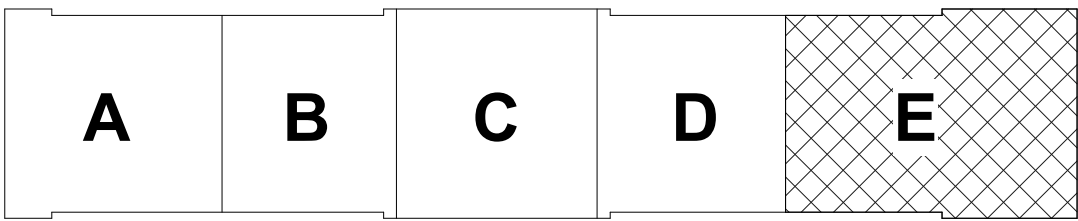


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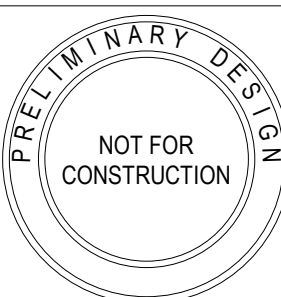
DESIGNED BY: A. VALENCIA	ISSUE DATE: NOVEMBER 13, 2025
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CHECKED BY: D. CLAYSON	CONTRACT NO.: W812PL25C0037
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SIZE: ANSI D	
US ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT	
KORTE CONSTRUCTION 5700 OAKLAND AVE, SUITE 275 ST. LOUIS, MO 63110	

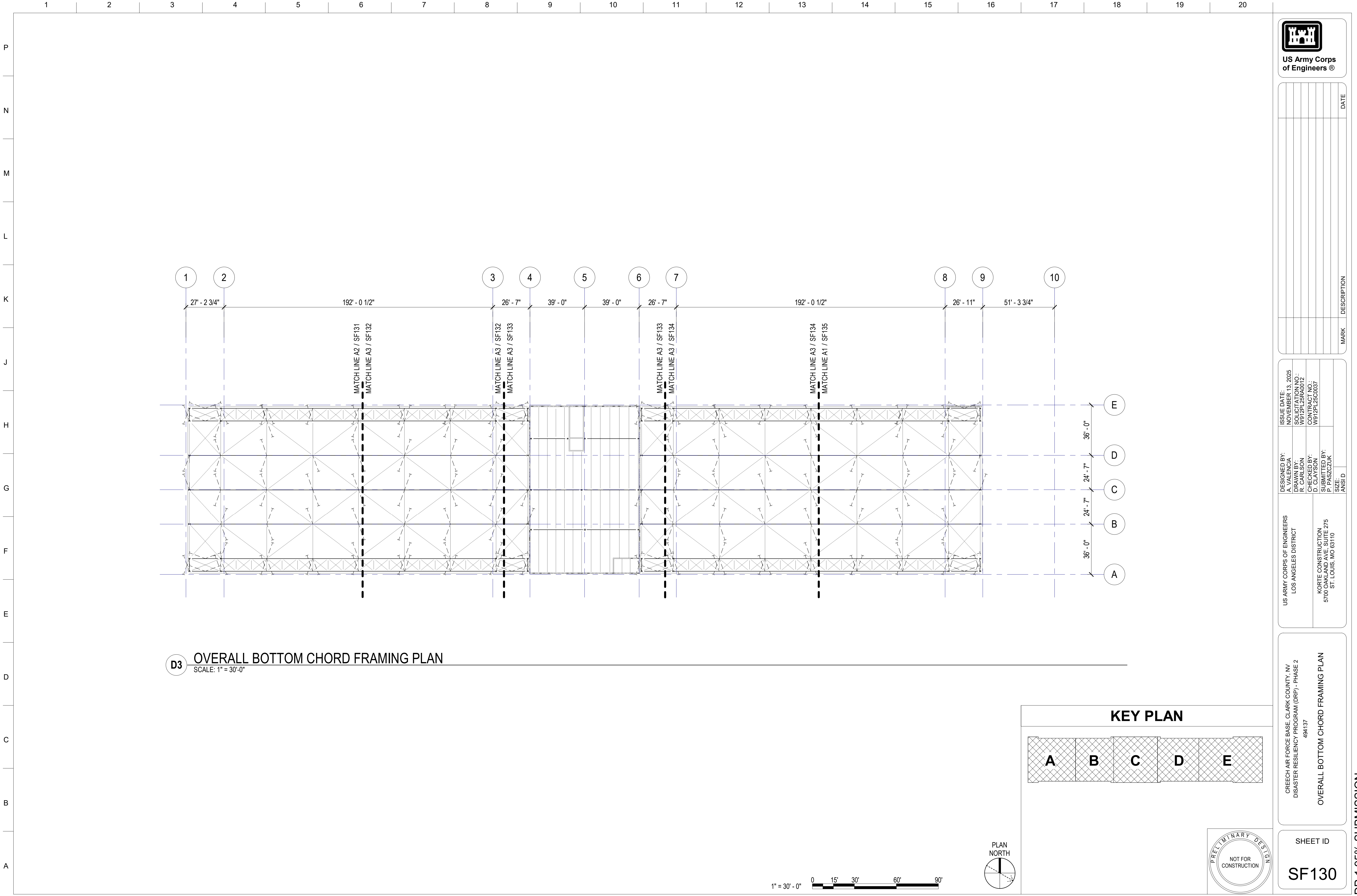
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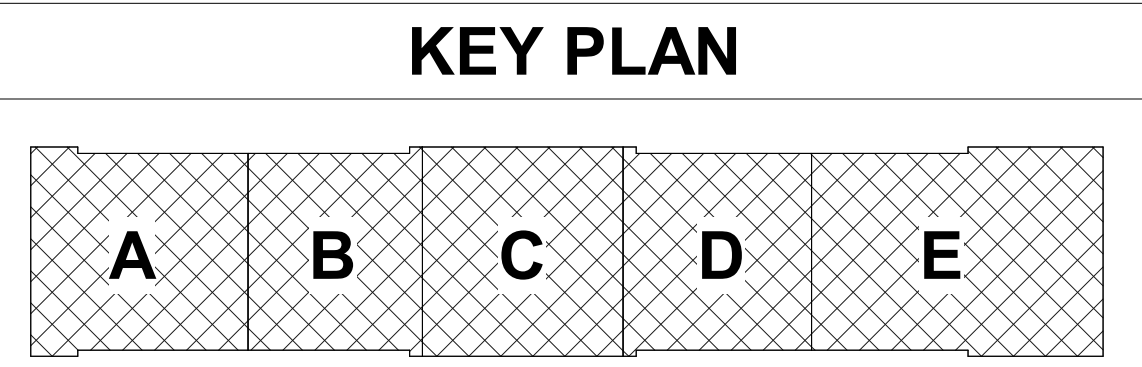
CREECH AIR FORCE BASE, CLARK COUNTY, NV
DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2
484.37
HANGAR DOOR FRAMING PLAN - AREA E

SHEET ID
SF129





D3 OVERALL BOTTOM CHORD FRAMING PLAN
SCALE: 1" = 30'-0"



**US Army Corps
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MARK		DESCRIPTION	DATE

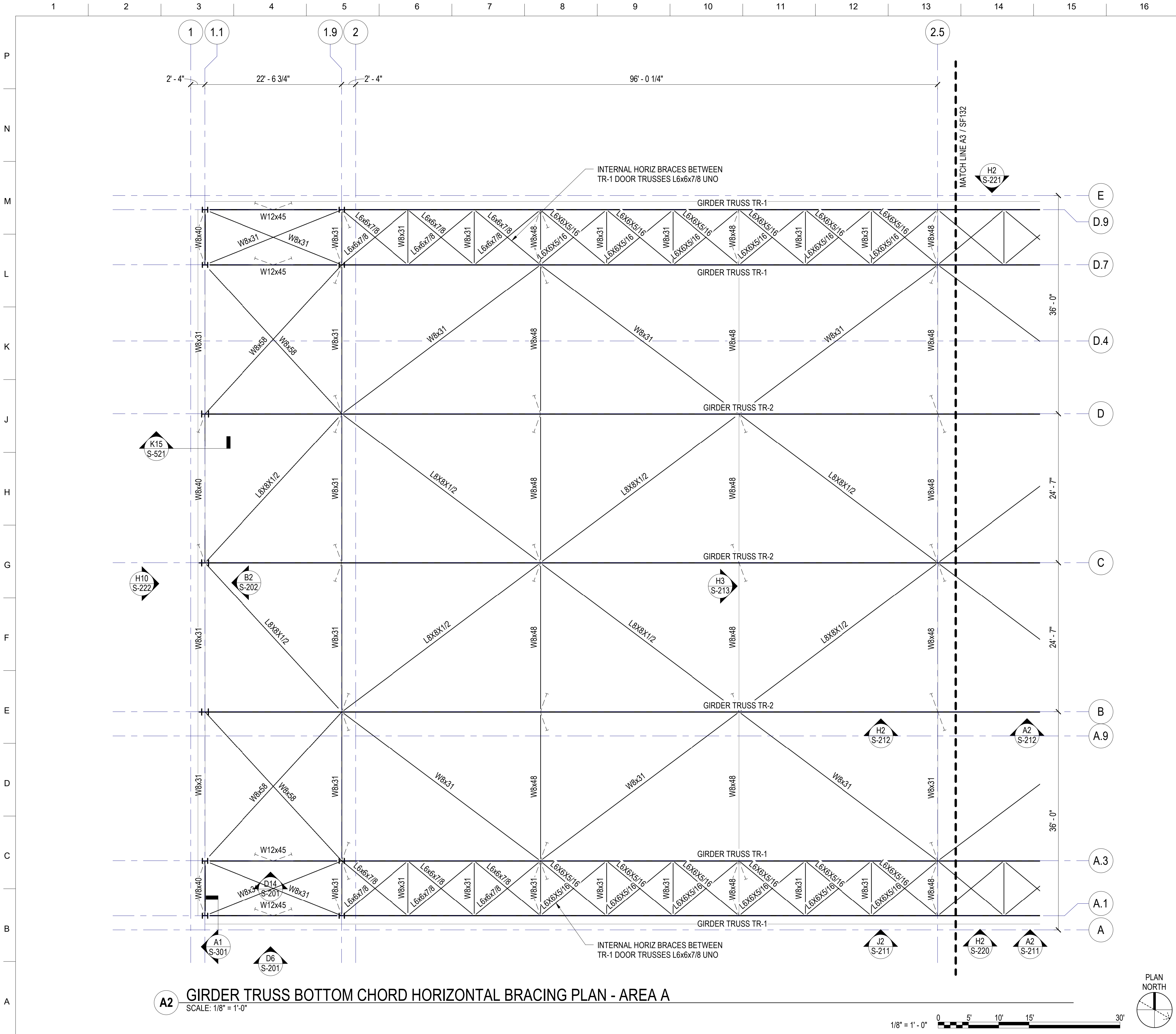
DESIGNED BY: A. VALENCIA		ISSUE DATE: NOVEMBER 13, 2025
DRAWN BY: R. CARLSON		SOLICITATION NO.: W812PL23RA0012
CHECKED BY: D. CLAYSON		CONTRACT NO.: W812PL25C0037
SUBMITTED BY: P. PASZCZUK		SIZE: ANSI D

US ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT	
KORTE CONSTRUCTION 5700 OAKLAND AVE. SUITE 275 ST. LOUIS, MO 63110	

CREECH AIR FORCE BASE, CLARK COUNTY, NV DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2 494137	
OVERALL BOTTOM CHORD FRAMING PLAN	

SHEET ID
SF130

DP-1 95% SUBMISSION



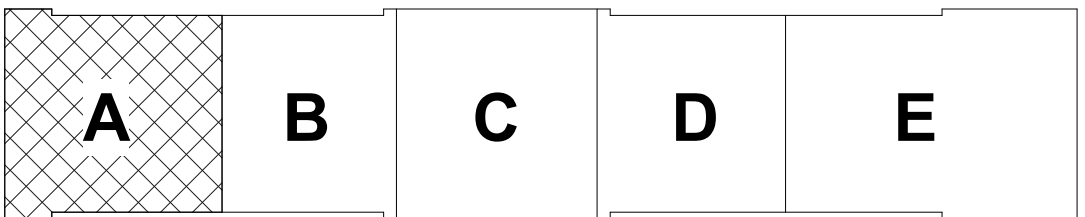
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4. RE: SHEETS S-001 – S-005 FOR GENERAL NOTES AND DESIGN CRITERIA.
5. RE: SHEETS S-511 - S-513 FOR TYPICAL ROOF FRAMING DETAILS.
6. RE: SHEETS S-601 - S-602 FOR SCHEDULES.

KEYNOTES

- ① PROVIDE L4x4x1/4 BRACING AT THIRD POINTS OF BEAM, RE: K15/S-521
- ② PROVIDE L2x2x1/4 BRACING AT THIRD POINTS OF BEAM, RE: K10/S-521
- ③ PROVIDE L8x8x1/2 BRACING AT THIRD POINTS OF BEAM, RE: E4/S-521
- ④ PROVIDE L8x8x1/2 BRACING AT BEAM HALF POINT, RE: E4/S-521 (SIM)
- ⑤ PROVIDE L8x8x1/2 BRACING AT THIRD POINTS OF BEAM, RE: A12/S-213

KEY PLAN



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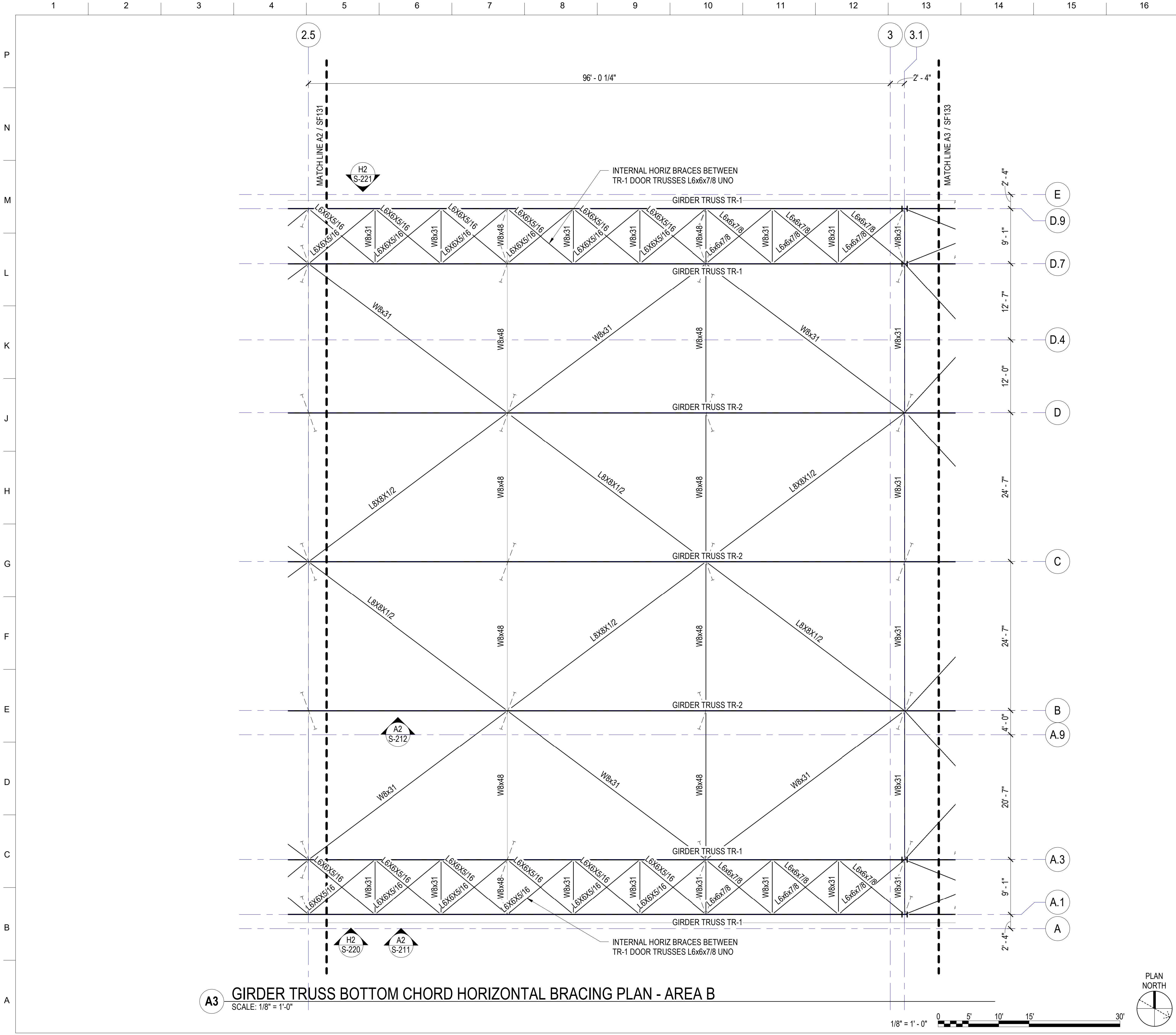
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CHECKED BY: D. CLAYSON	CONTRACT NO.: W912PL25C0037
SUBMITTED BY: P. PASZCZUK	SIZE: ANSI D
US ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT	
KORTE CONSTRUCTION 5700 OAKLAND AVE. SUITE 275 ST. LOUIS, MO 63110	

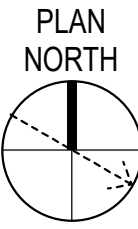
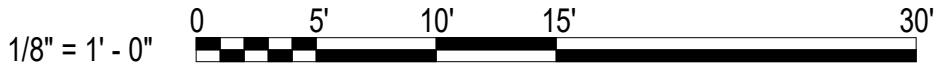
CREECH AIR FORCE BASE, CLARK COUNTY, NV DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2 494.37	BOTTOM CHORD FRAMING PLAN - AREA A
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SHEET ID
SF131

DP-1 95% SUBMISSION



A3 GIRDER TRUSS BOTTOM CHORD HORIZONTAL BRACING PLAN - AREA B
SCALE: 1/8" = 1'-0"



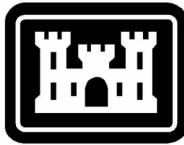
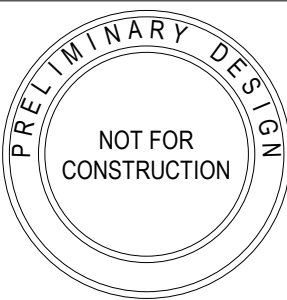
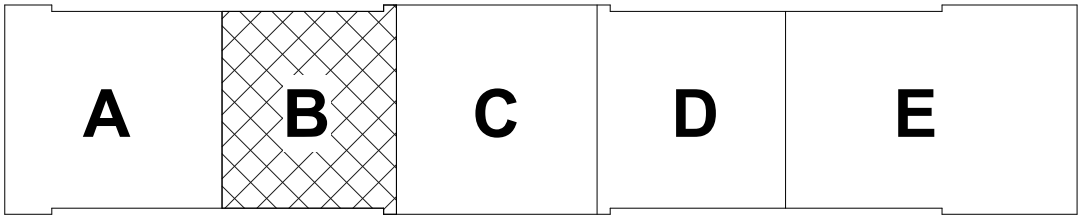
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- ④ PROVIDE L8x8x1/2 BRACING AT BEAM HALF POINT, RE: E4/S-521 (SIM)
- ⑤ PROVIDE L8x8x1/2 BRACING AT THIRD POINTS OF BEAM, RE: A12/S-213

KEY PLAN



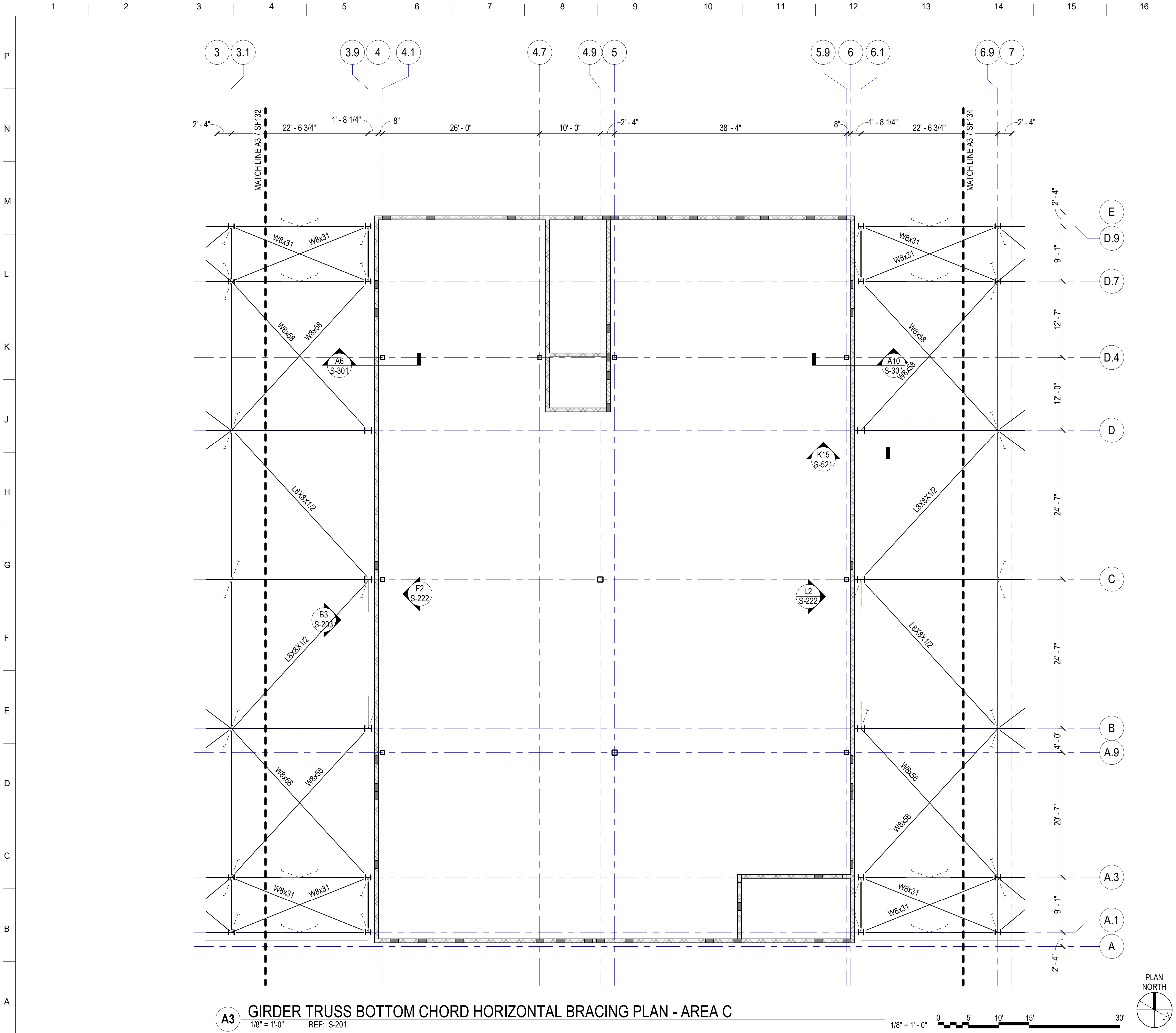
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CHECKED BY: D. CLAYSON	CONTRACT NO.: WB12PL25C0037
SUBMITTED BY: P. PASZCZUK	
SIZE: ANSI D	
US ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT	
KORTE CONSTRUCTION 5700 OAKLAND AVE. SUITE 275 ST. LOUIS, MO 63110	

CREECH AIR FORCE BASE, CLARK COUNTY, NV DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2 494137 BOTTOM CHORD FRAMING PLAN - AREA B
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SHEET ID SF132

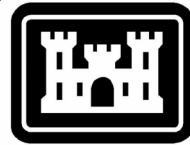


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- ④ PROVIDE L8x8x1/2 BRACING AT BEAM HALF POINT, RE: E4/S-521 (SIM)
- ⑤ PROVIDE L8x8x1/2 BRACING AT THIRD POINTS OF BEAM, RE: A12/S-213



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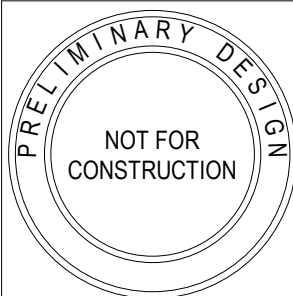
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DRAWN BY: R. CARLSON	SOLICITATION NO.: WB12PL23RA0012
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SUBMITTED BY: P. PASZCZUK	
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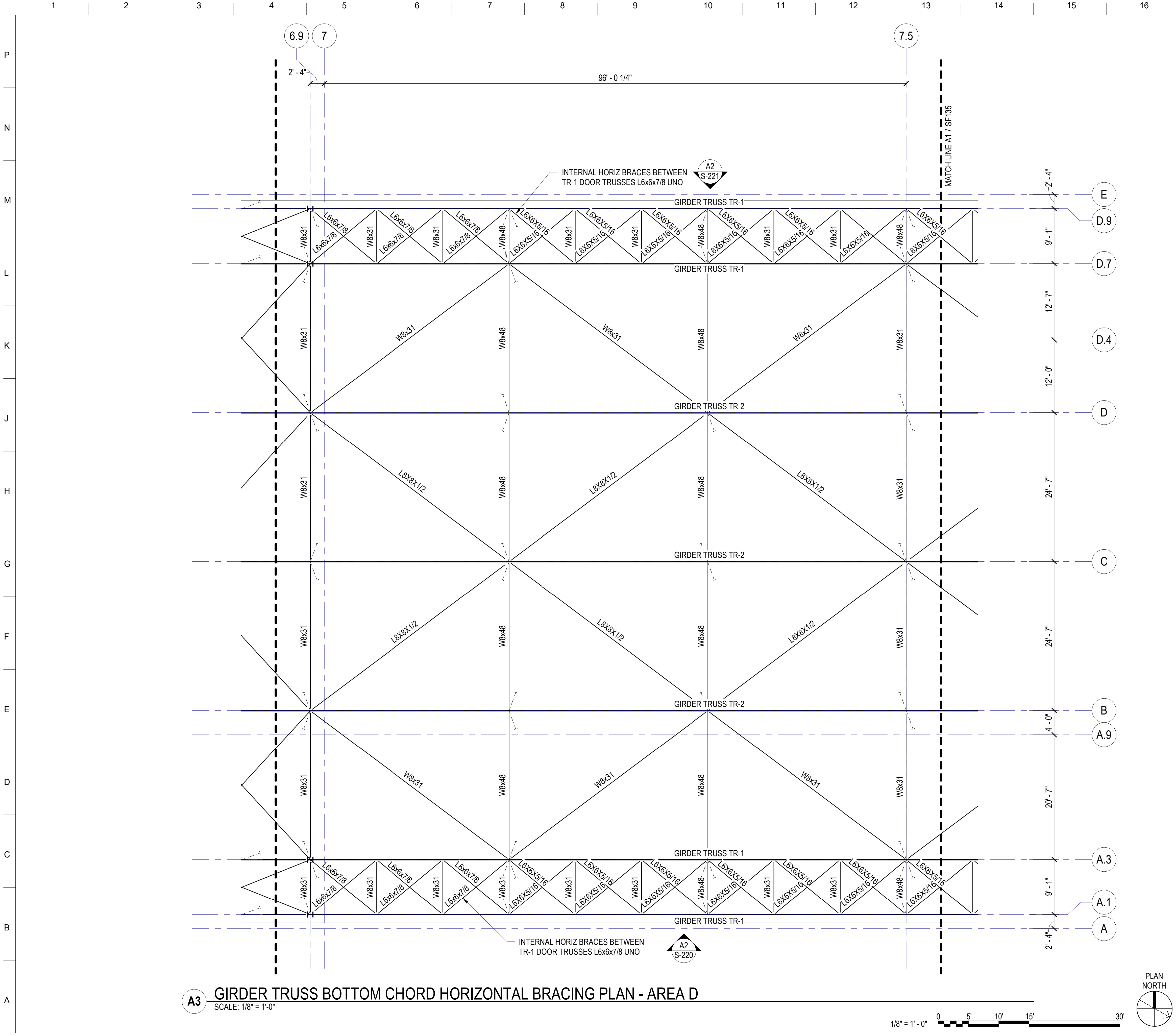
US ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT	KORTE CONSTRUCTION 5700 OAKLAND AVE, SUITE 275 ST. LOUIS, MO 63110
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CREECH AIR FORCE BASE, CLARK COUNTY, NV DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2 494137	BOTTOM CHORD FRAMING PLAN - AREA C
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SHEET ID
SF133



DP-1 95% SUBMISSION



FRAMING NOTES

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MARK	DESCRIPTION	DATE

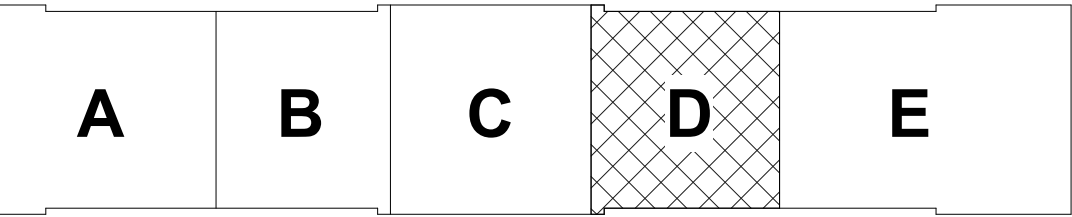
DESIGNED BY: A. VALENCIA	ISSUE DATE: NOVEMBER 13, 2025
DRAWN BY: R. CARLSON	SOLICITATION NO.: WB12PL23RA0012
CHECKED BY: D. CLAYSON	CONTRACT NO.: WB12PL25C0037
SUBMITTED BY: P. PASZCZUK	
SIZE: ANSI D	

US ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT	KORTE CONSTRUCTION 5700 OAKLAND AVE, SUITE 275 ST. LOUIS, MO 63110
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KEYNOTES

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- ④ PROVIDE L8x8x1/2 BRACING AT BEAM HALF POINT, RE: E4/S-521 (SIM)
- ⑤ PROVIDE L8x8x1/2 BRACING AT THIRD POINTS OF BEAM, RE: A12/S-213

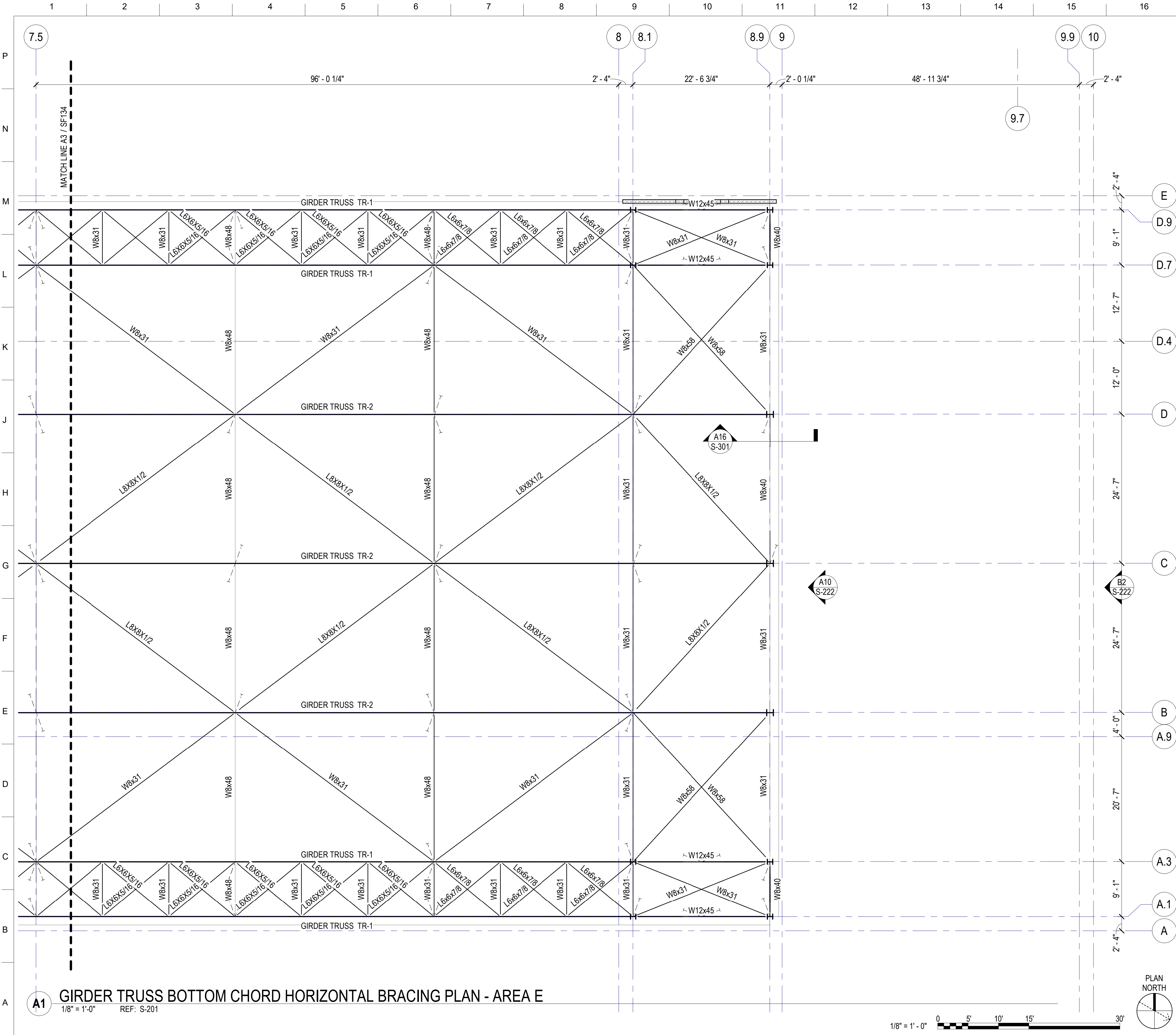
KEY PLAN



CREECH AIR FORCE BASE, CLARK COUNTY, NV
DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2
494.37
BOTTOM CHORD FRAMING PLAN - AREA D

SHEET ID
SF134

DP-1 95% SUBMISSION



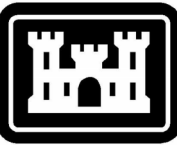
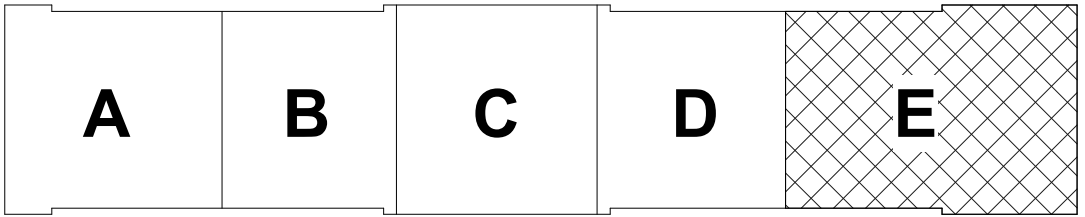
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KEY PLAN



US Army Corps
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DATE	DESCRIPTION	MARK

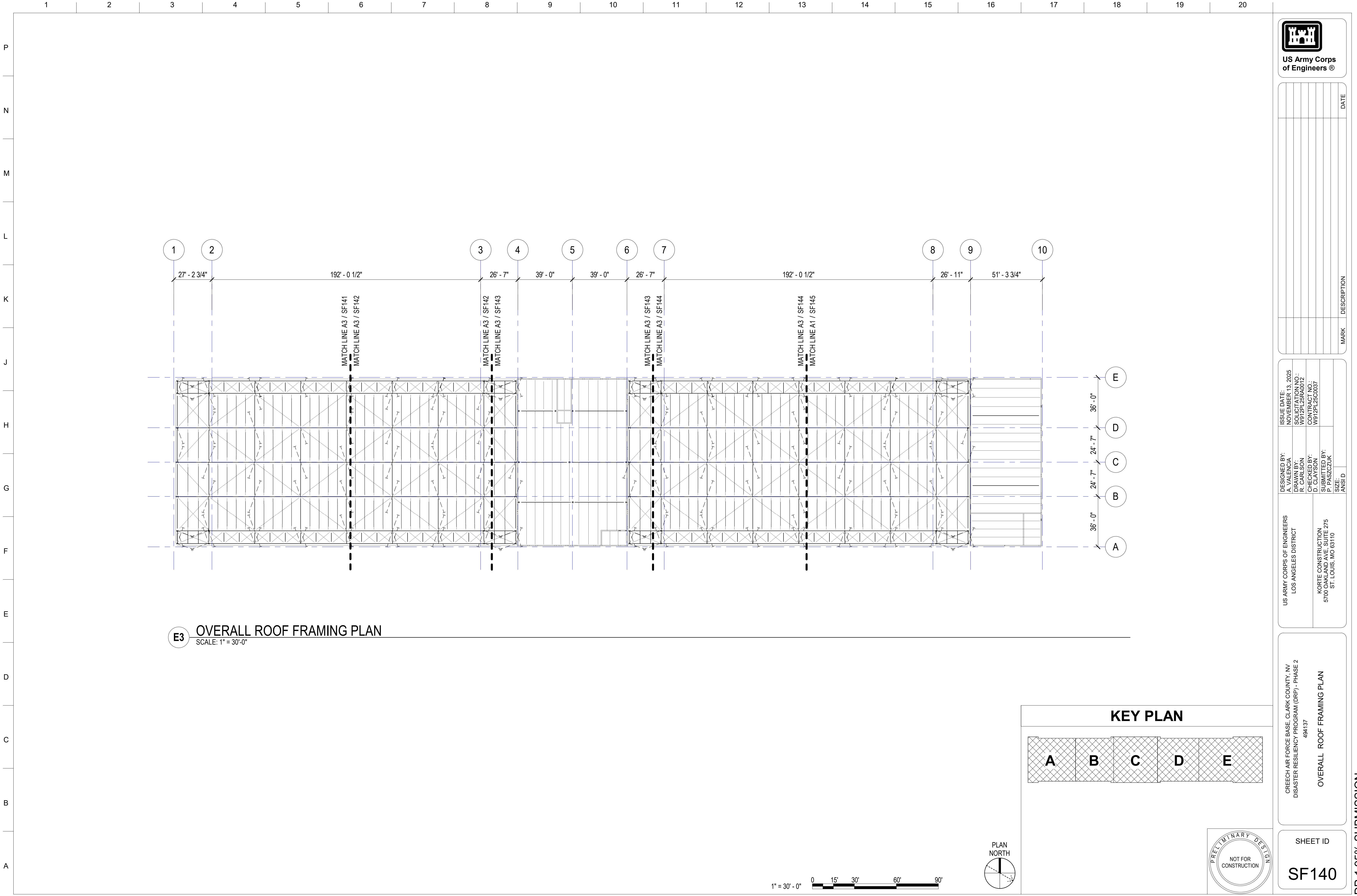
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US ARMY CORPS OF ENGINEERS
LOS ANGELES DISTRICT

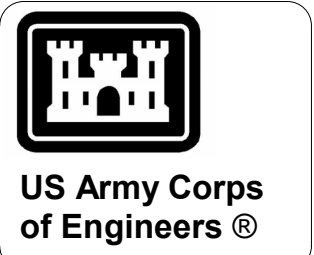
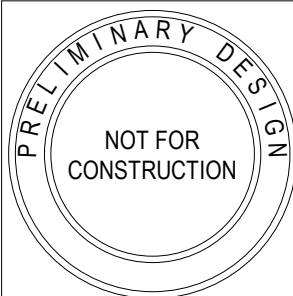
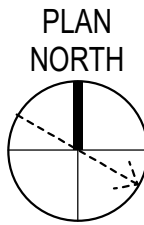
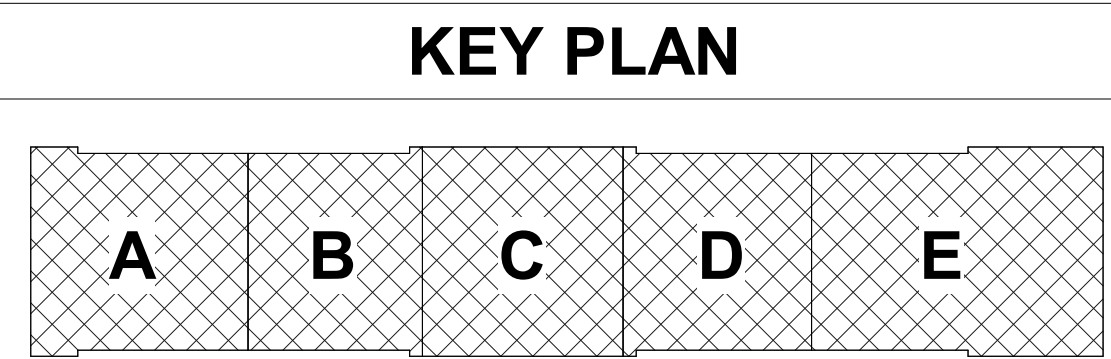
KORTE CONSTRUCTION
5700 OAKLAND AVE, SUITE 275
ST. LOUIS, MO 63110

CREECH AIR FORCE BASE, CLARK COUNTY, NV DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2 494.37	BOTTOM CHORD FRAMING PLAN - AREA E
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SHEET ID
SF135



E3 OVERALL ROOF FRAMING PLAN
SCALE: 1" = 30'-0"

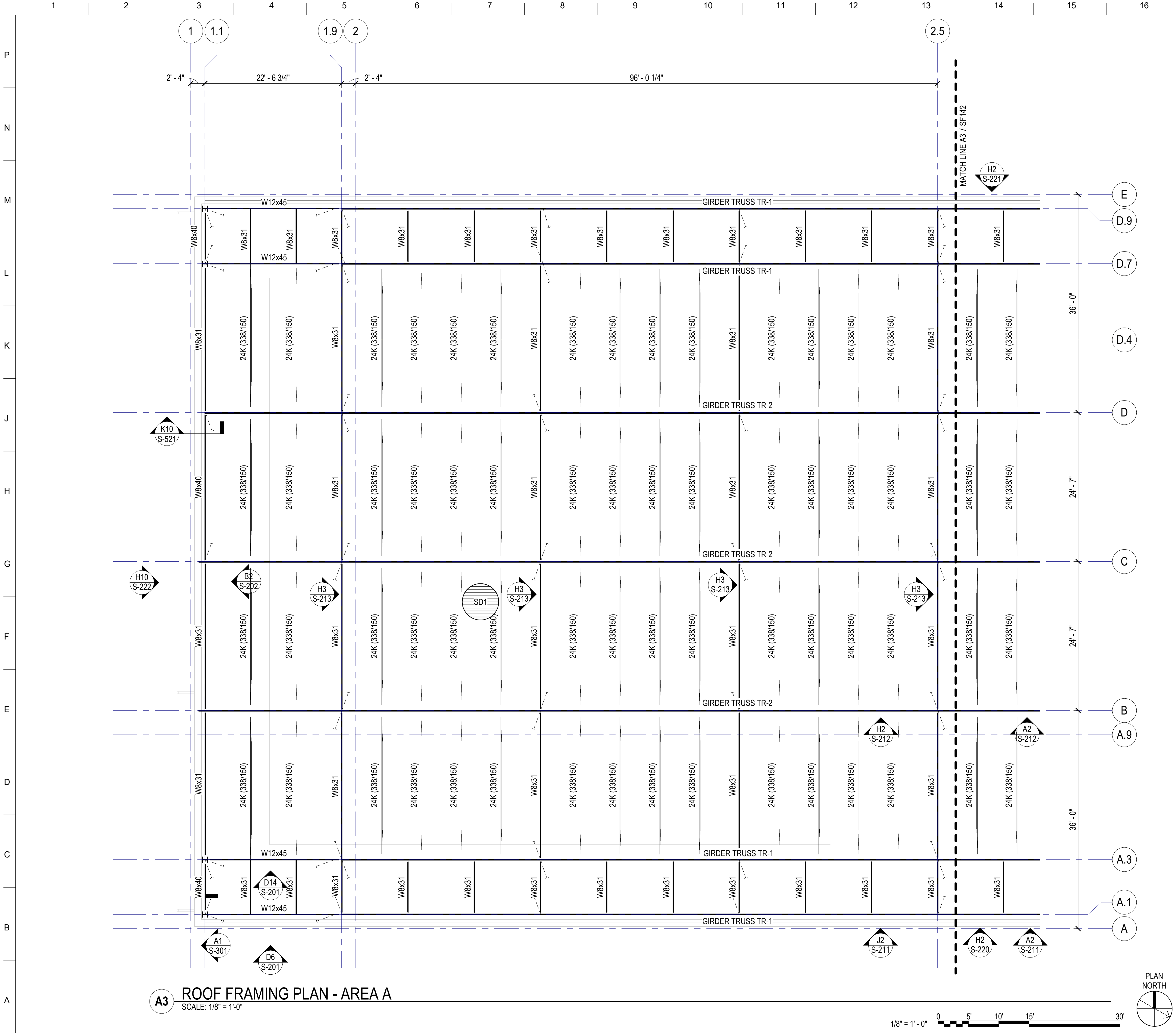


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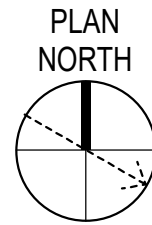
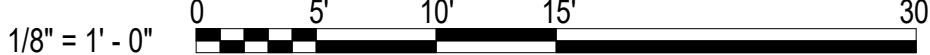
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US ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT	
KORTE CONSTRUCTION 5700 OAKLAND AVE, SUITE 275 ST. LOUIS, MO 63110	

CREECH AIR FORCE BASE, CLARK COUNTY, NV DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2 494.37	OVERALL ROOF FRAMING PLAN
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SHEET ID SF140



A3 ROOF FRAMING PLAN - AREA A
SCALE: 1/8" = 1'-0"



ROOF FRAMING NOTES

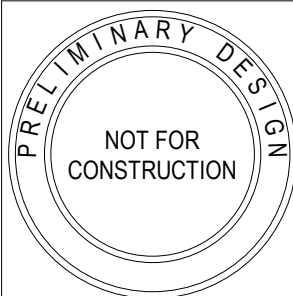
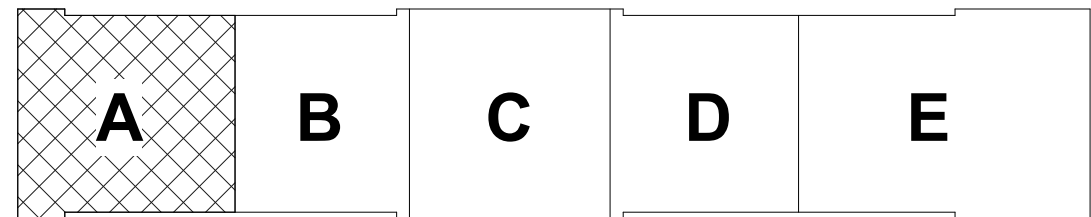
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2. ALL BEAM-TO-COLUMN CONNECTIONS SHALL BE SINGLE SHEAR TAB CONNECTIONS UNLESS NOTED OTHERWISE. SEE TYPICAL STEEL-TO-STEEL CONNECTION SCHEDULE.
3. SHOP DRAWINGS MUST BE PRODUCED FOR ALL JOISTS AND SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO FABRICATION. CALCULATIONS SHALL BE SUBMITTED WITH THE SHOP DRAWINGS AND SHALL BEAR THE STAMP OF A LICENSED ENGINEER.
4. OPEN WEB STEEL JOISTS SHALL BE DESIGNED BY THE MANUFACTURER TO SUPPORT THE MECHANICAL, AXIAL (WL/EQ), LATERAL, POINT, AND UNIFORM LOADS SHOWN ON PLANS AND IN DETAILS. LOADS SHOWN ARE ASD MAGNITUDE LOADS UNLESS NOTED OTHERWISE.
5. THE STEEL JOIST SUPPLIER IS RESPONSIBLE IN DESIGNING ALL JOISTS, INCLUDING SLOPE, CAMBER, BEARING ENDS, ETC. ALL DESIGNS SHALL BE IN ACCORDANCE WITH SJI SPECIFICATIONS WITH A MAXIMUM DEFLECTION OF JOISTS OF TL/180 AND LL/240.
6. HORIZONTAL AND CROSS BRIDGING SHALL BE SIZED, LOCATED, AND SUPPLIED BY THE JOIST MANUFACTURER.
7. ALL STABILIZER PLATES SHALL BE 6"x6"xCHORD GAP-1/4" WITH A 3/4" DIAMETER HOLE, AND MUST EXTEND 3" MINIMUM BELOW THE BOTTOM CHORD.
8. ALL CONCENTRATED LOADS GREATER THAN 100 POUNDS SUPPORTED BY OPEN WEB JOISTS SHALL BE LOCATED WITHIN 6" OF PANEL POINT. OTHERWISE, JOIST SHALL BE REINFORCED WITH AN ADDITIONAL WEB MEMBER. (RE: H1/S-511)
9. BOTTOM CHORD OF ANY JOIST SHALL NOT BE USED TO BRACE ANY MISC EQUIPMENT.
10. JOIST BRIDGING SHALL NEVER BE USED TO SUPPORT HANGING LOADS.
11. JOIST LOADS SHOWN ON PLAN WITH THE JOIST DESIGNATION "K (XX/XX)" ACCOUNTS FOR TOTAL LOAD/LIVE LOAD (ASD).
12. OPEN WEB STEEL JOISTS SHALL BE DESIGNED FOR A NET UPLIFT DUE TO WIND LOADING, RE: SHEET S-005.
13. STEEL JOISTS SHALL BE DESIGNED FOR AN ADDITIONAL VERT LIVE LOAD (ASD) OF +/-500# AT EACH PANEL POINT
14. STANDARD JOIST SEAT DEPTH SHALL BE 2 1/2". CONTRACTOR TO COORDINATE TOP OF GIRDERS USING 2 1/2" JOIST SEAT DEPTH UNO.
15. CONTRACTOR TO COORDINATE ALL ROOF EDGES AND OPENINGS. OPENINGS LESS THAN 6" IN ALL DIRECTIONS ARE NOT SHOWN IN STRUCTURAL DRAWINGS. RE: L1-L17/S-511 FOR PLACEMENT CRITERIA.
16. RE: SHEETS S-001 - S-005 FOR GENERAL NOTES AND DESIGN CRITERIA.
17. RE: SHEETS S-511 - S-513 FOR TYPICAL ROOF FRAMING DETAILS.
18. RE: SHEETS S-601 - S-602 FOR SCHEDULES.

KEYNOTES

- ① HANGING FAN ON BOTTOM CHORD OF TRUSS ON GRID D. APPROXIMATE WEIGHT OF 300LBS (DL ASD). RE: ARCH/MEP FOR EXACT LOCATION. RE: MFR FOR ATTACHMENT
- ② PROVIDE L4x4x1/4 BRACING AT THIRD POINTS OF BEAM
- ③ PROVIDE L8x8x1/2 BRACING AT THIRD POINTS OF BEAM
- ④ PROVIDE L8x8x1/2 BRACING AT BEAM HALF POINT
- ⑤ SUPPORT FRAMING FOR 1-TON MONORAIL HOIST CRANE

**CRANE COORDINATION AND
LATERAL BRACING IN PROGRESS**

KEY PLAN



**US Army Corps
of Engineers®**

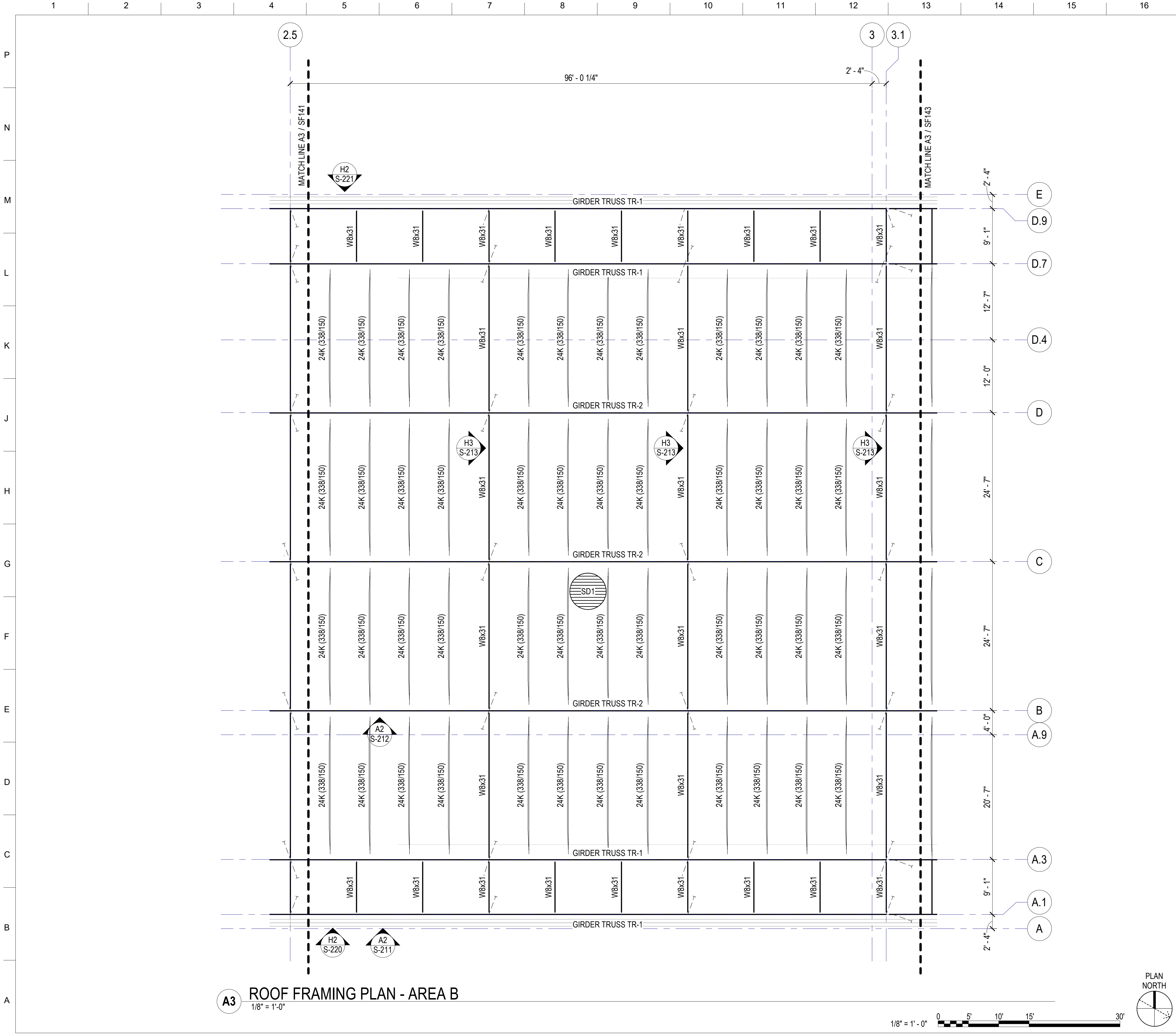
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	SOLICITATION NO.: W912PL29RA0012	
	DRAWN BY: R. CARLSON	
	CHECKED BY: D. CLAYSON	
US ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT	CONTRACT NO.: W912PL25C0037	
	SUBMITTED BY: P. PASZCZUK	
	KORTE CONSTRUCTION 5700 OAKLAND AVE, SUITE 275 ST. LOUIS, MO 63110	
	SIZE: ANSI D	

CREECH AIR FORCE BASE, CLARK COUNTY, NV DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2 494137	ROOF FRAMING PLAN - AREA A

SHEET ID

SF141



A3 ROOF FRAMING PLAN - AREA B
1/8" = 1'-0"

ROOF FRAMING NOTES

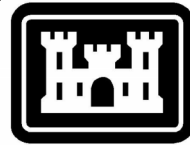
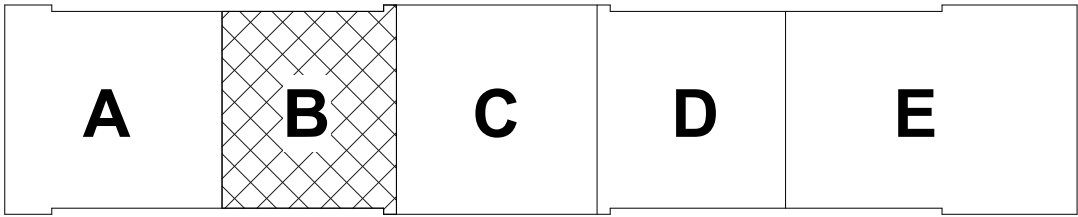
- CONTRACTOR IS RESPONSIBLE FOR THE CONSTRUCTION SEQUENCE FOR ALL STRUCTURAL ELEMENTS IN THE PROJECT. CONTRACTOR IS RESPONSIBLE TO PROVIDE ANY SHORING OR BRACING AS NEEDED UNTIL STRUCTURE IS COMPLETE.
- ALL BEAM-TO-COLUMN CONNECTIONS SHALL BE SINGLE SHEAR TAB CONNECTIONS UNLESS NOTED OTHERWISE. SEE TYPICAL STEEL-TO-STEEL CONNECTION SCHEDULE.
- SHOP DRAWINGS MUST BE PRODUCED FOR ALL JOISTS AND SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO FABRICATION. CALCULATIONS SHALL BE SUBMITTED WITH THE SHOP DRAWINGS AND SHALL BEAR THE STAMP OF A LICENSED ENGINEER.
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- RE: SHEETS S-001 - S-005 FOR GENERAL NOTES AND DESIGN CRITERIA.
- RE: SHEETS S-511 - S-513 FOR TYPICAL ROOF FRAMING DETAILS.
- RE: SHEETS S-601 - S-602 FOR SCHEDULES.

KEYNOTES

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- PROVIDE L8x8x1/2 BRACING AT THIRD POINTS OF BEAM
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- SUPPORT FRAMING FOR 1-TON MONORAIL HOIST CRANE

**CRANE COORDINATION AND
LATERAL BRACING IN PROGRESS**

KEY PLAN



**US Army Corps
of Engineers®**

MARK	DESCRIPTION	DATE

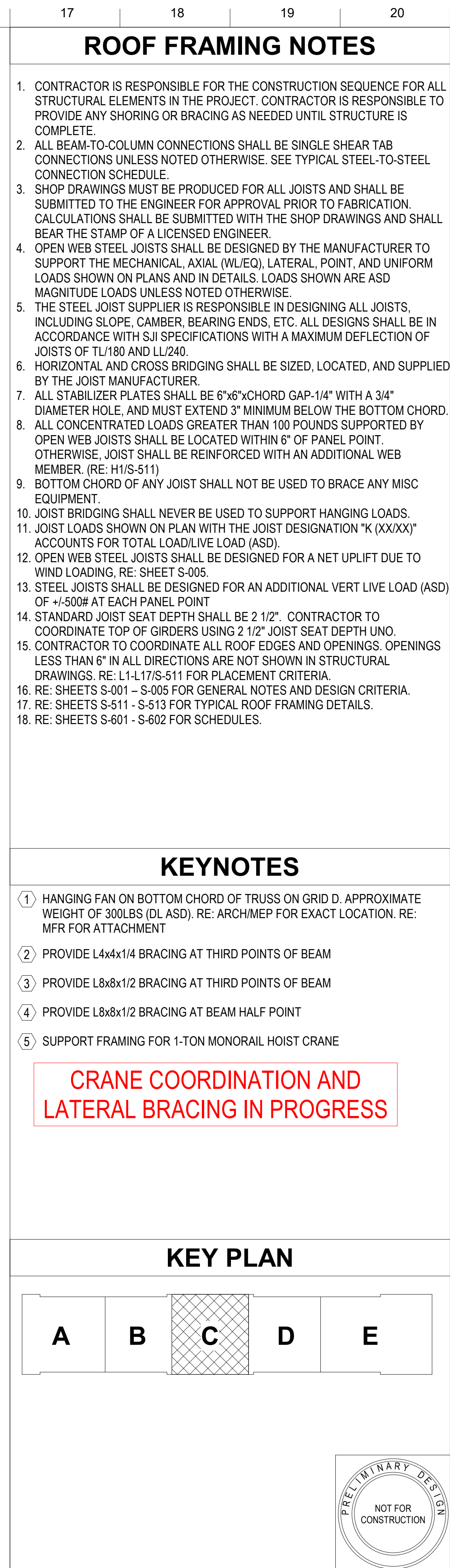
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DRAWN BY: R. CARLSON	SOLICITATION NO.: W812PL23RA0012
CHECKED BY: D. CLAYSON	CONTRACT NO.: W812PL25C0037
SUBMITTED BY: P. PASZCZUK	
SIZE: ANSI D	

US ARMY CORPS OF ENGINEERS
LOS ANGELES DISTRICT

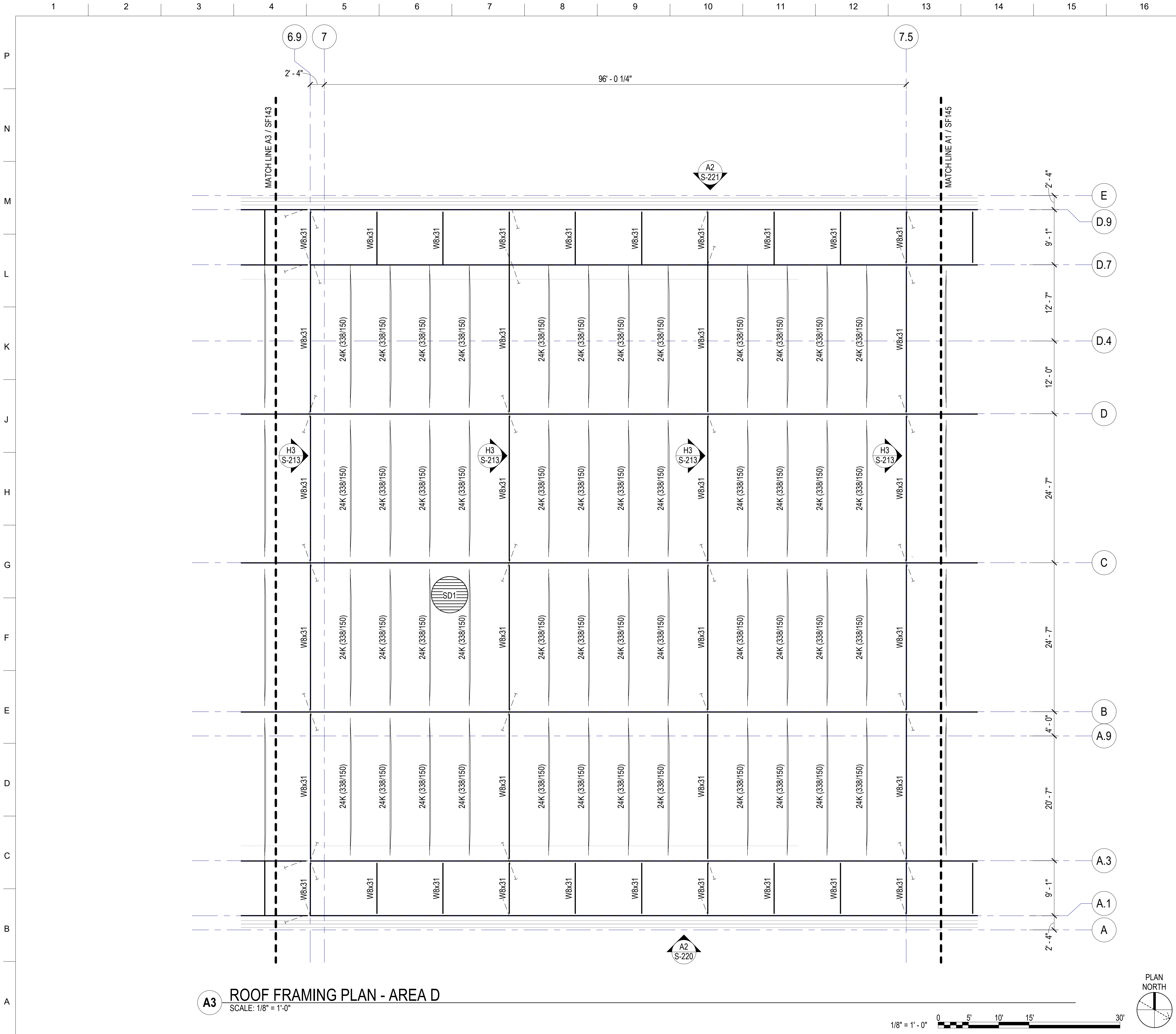
KORTE CONSTRUCTION
5700 OAKLAND AVE, SUITE 275
ST. LOUIS, MO 63110

CREECH AIR FORCE BASE, CLARK COUNTY, NV DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2 494137	ROOF FRAMING PLAN - AREA B
--	----------------------------

SHEET ID SF142



DP-1 95% SUBMISSION



ROOF FRAMING NOTES

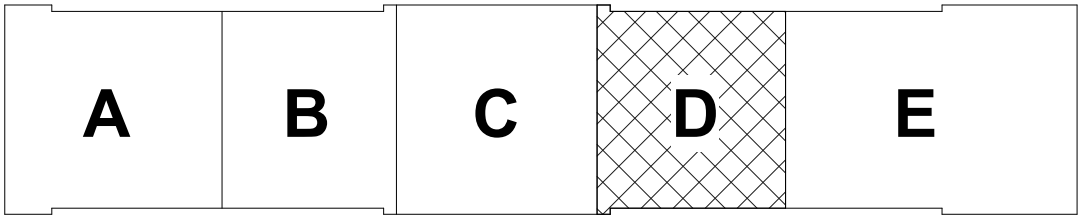
- CONTRACTOR IS RESPONSIBLE FOR THE CONSTRUCTION SEQUENCE FOR ALL STRUCTURAL ELEMENTS IN THE PROJECT. CONTRACTOR IS RESPONSIBLE TO PROVIDE ANY SHORING OR BRACING AS NEEDED UNTIL STRUCTURE IS COMPLETE.
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- RE: SHEETS S-601 - S-602 FOR SCHEDULES.

KEYNOTES

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- SUPPORT FRAMING FOR 1-TON MONORAIL HOIST CRANE

CRANE COORDINATION AND
LATERAL BRACING IN PROGRESS

KEY PLAN

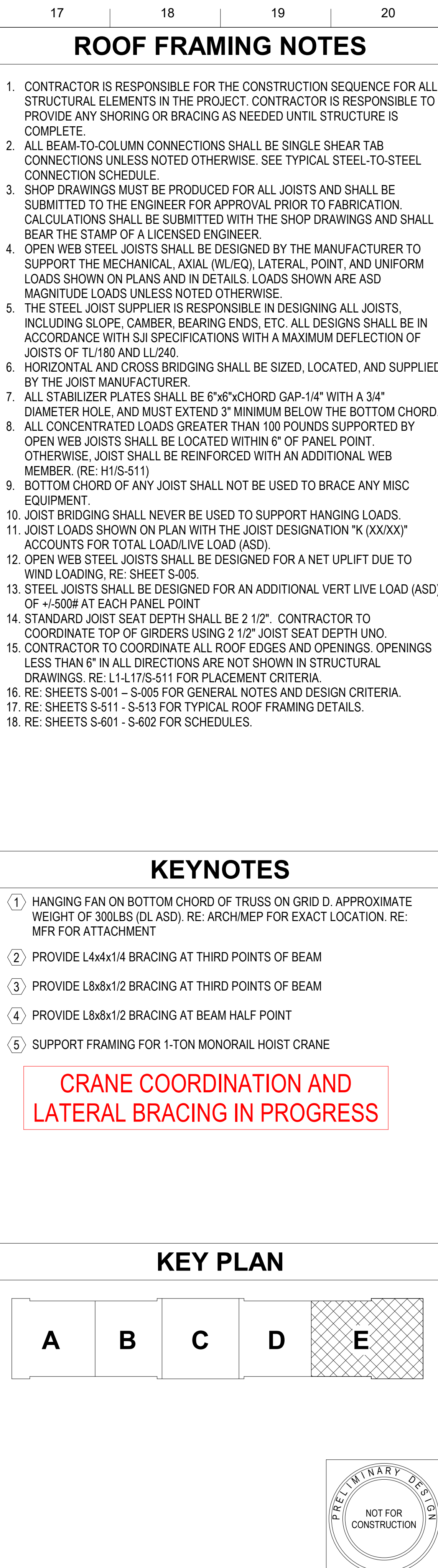


DATE	DESCRIPTION	MARK

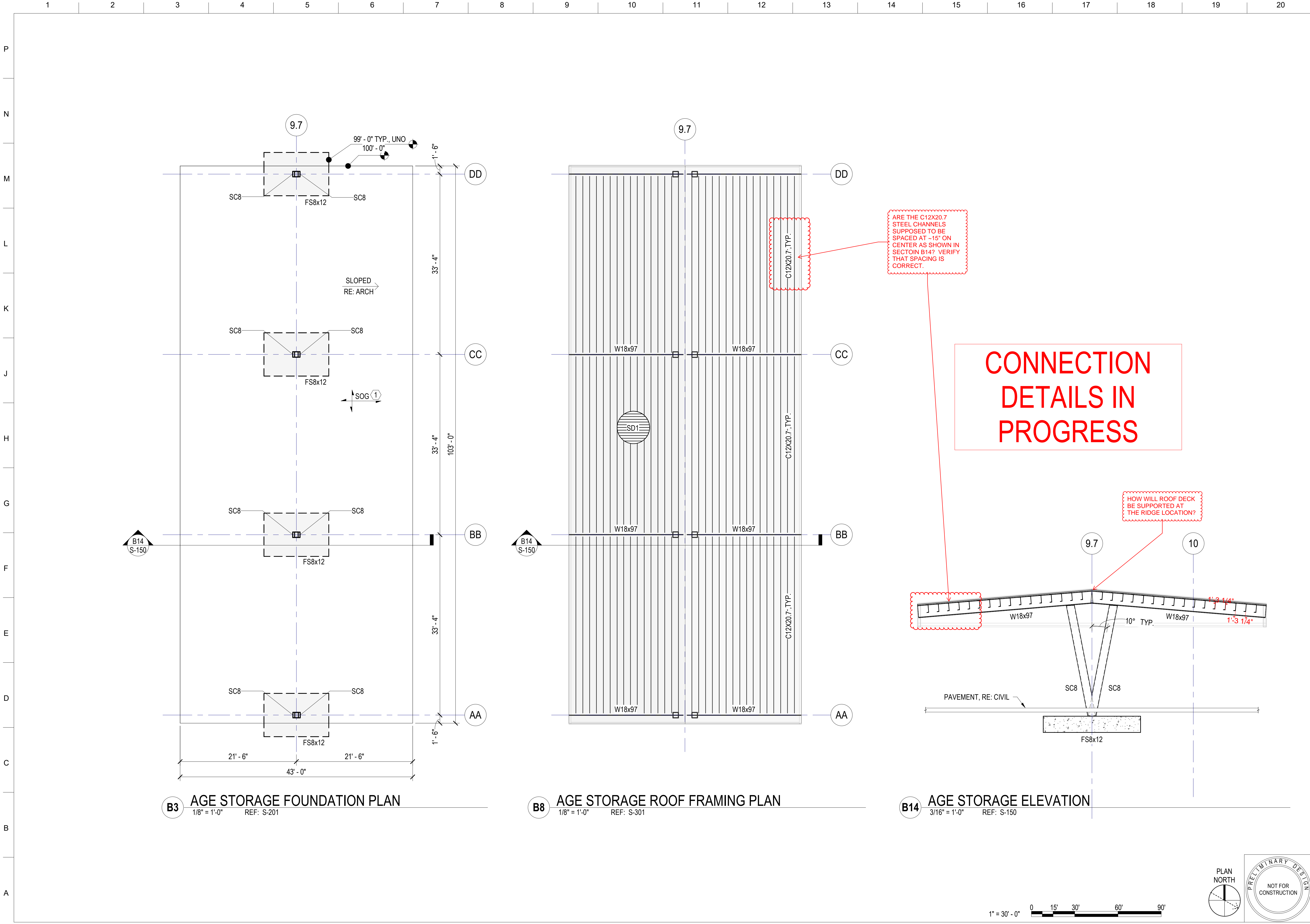
DESIGNED BY: A. VALENCIA	ISSUE DATE: NOVEMBER 13, 2025
DRAWN BY: R. CARLSON	SOLICITATION NO.: WB12PL23RA0012
CHECKED BY: D. CLAYSON	CONTRACT NO.: WB12PL25C0037
SUBMITTED BY: P. PASZCZUK	SIZE: ANSI D
US ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT	
KORTE CONSTRUCTION 5700 OAKLAND AVE, SUITE 275 ST. LOUIS, MO 63110	

CREECH AIR FORCE BASE, CLARK COUNTY, NV DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2 494.37	ROOF FRAMING PLAN - AREA D
--	----------------------------

SHEET ID
SF144



DP-1 95% SUBMISSION



US Army Corps
of Engineers®

DATE		DESCRIPTION		MARK	

DESIGNED BY:	ISSUE DATE:
A. VALENCIA	NOVEMBER 13, 2025
DRAWN BY:	SOLICITATION NO.:
R. CARLSON	W912PL25RA0012
CHECKED BY:	CONTRACT NO.:
D. CLAYSON	W912PL25C0037
SUBMITTED BY:	
P. PASZCZUK	
SIZE:	ANSI D

US ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT	KORTE CONSTRUCTION 5700 OAKLAND AVE, SUITE 275 ST. LOUIS, MO 63110
--	--

CREECH AIR FORCE BASE, CLARK COUNTY, NV DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2 494.37	AGE STORAGE PLAN & ELEVATION
--	------------------------------

SHEET ID
S-150

PRELIMINARY DESIGN

NOT FOR CONSTRUCTION

PLAN NORTH

1" = 30' - 0"

DP-1 95% SUBMISSION



**US Army Corps
of Engineers®**

[illegible]

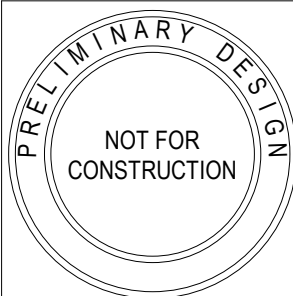
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	CHECKED BY: R. CARLSON	CONTRACT NO.: W11P125RA0012
	DRAWN BY: D. CLAYTON	CONTRACT NO.: W11P125C00037
	SUBMITTED BY: J. ANSELZUK	
KORTE CONSTRUCTION 5700 OAKLAND AVE, SUITE 275 ST. LOUIS, MO 63110	SIZE: ANSI D	

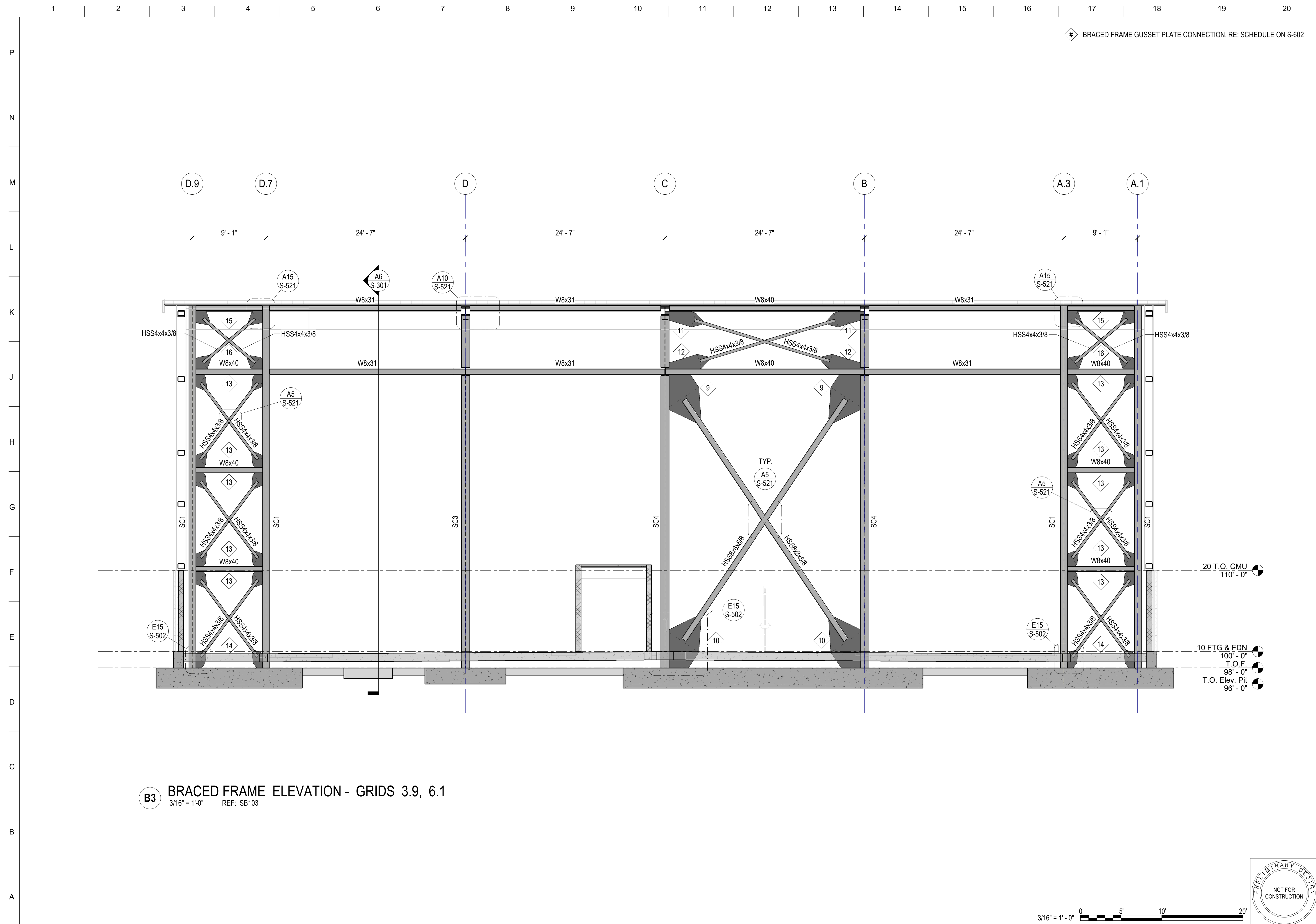
GREECH AIR FORCE BASE, CLARK COUNTY, NV
DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2
494137

SHEET ID

S-201

DP-1 95% SUBMISSION





**S Army Corps
f Engineers ®**

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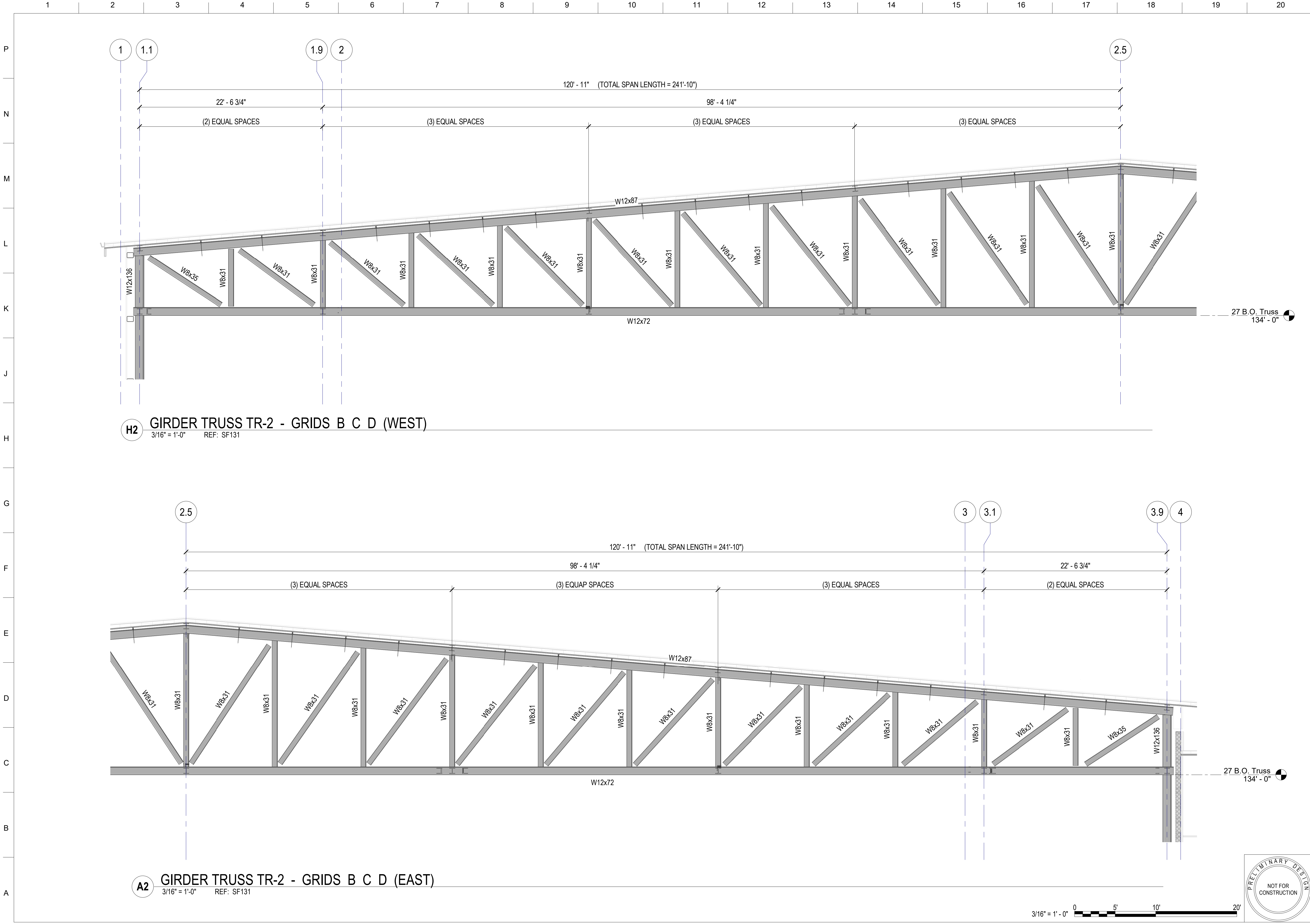
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BRACED FRAME ELEVATIONS

SHEET ID

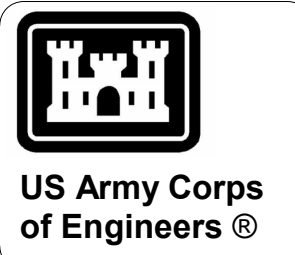
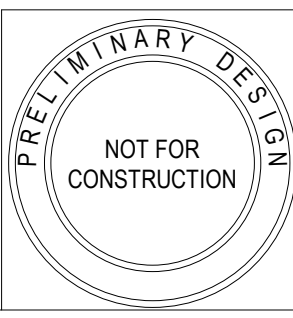
S-203

DP-1 95% SUBMISSION



H2 GIRDERS TRUSS TR-2 - GRIDS B C D (WEST)
3/16" = 1'-0" REF: SF131

A2 GIRDERS TRUSS TR-2 - GRIDS B C D (EAST)
3/16" = 1'-0" REF: SF131

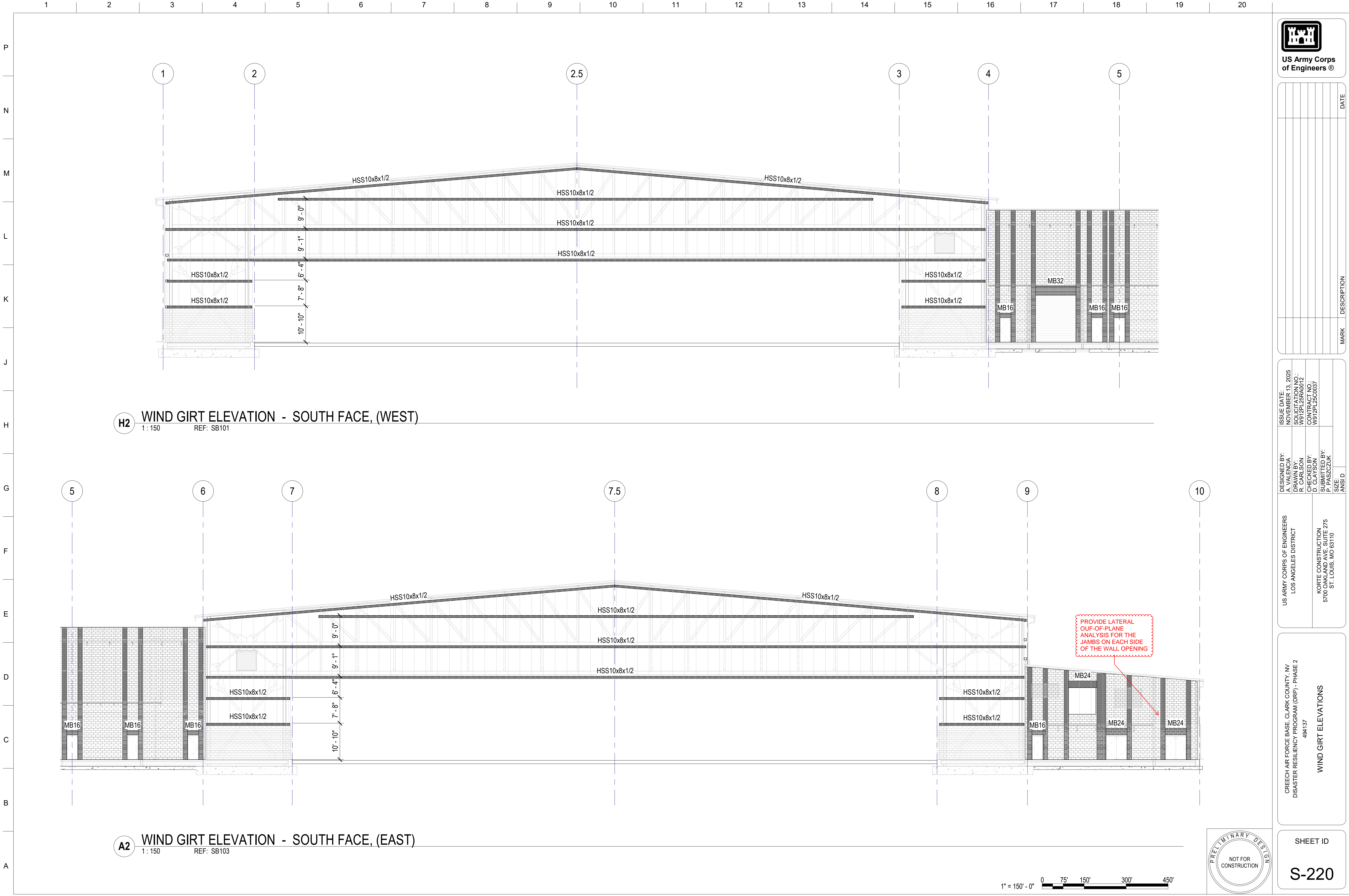


MARK	DESCRIPTION	DATE

DESIGNED BY: A. VALENCIA	ISSUE DATE: NOVEMBER 13, 2025
DRAWN BY: R. CARLSON	SOLICITATION NO.: W912PL25RA0012
CHECKED BY: D. CLAYSON	CONTRACT NO.: W912PL25C0037
SUBMITTED BY: P. PASZCZUK	SIZE: ANSI D
US ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT	
KORTE CONSTRUCTION 5700 OAKLAND AVE. SUITE 275 ST. LOUIS, MO 63110	

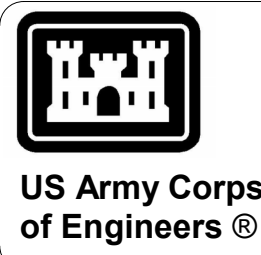
CREECH AIR FORCE BASE, CLARK COUNTY, NV DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2 494.37	GIRDERS TRUSS ELEVATIONS
--	--------------------------

SHEET ID S-212



H2 WIND GIRT ELEVATION - SOUTH FACE, (WEST)
1 : 150 REF: SB101

A2 WIND GIRT ELEVATION - SOUTH FACE, (EAST)
1 : 150 REF: SB103

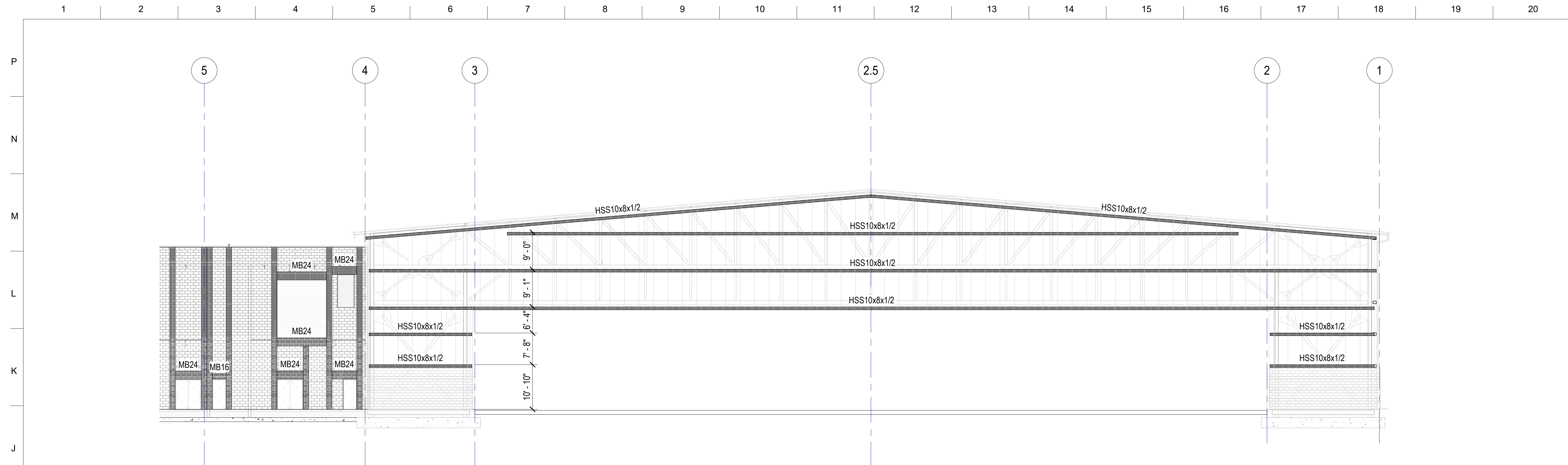


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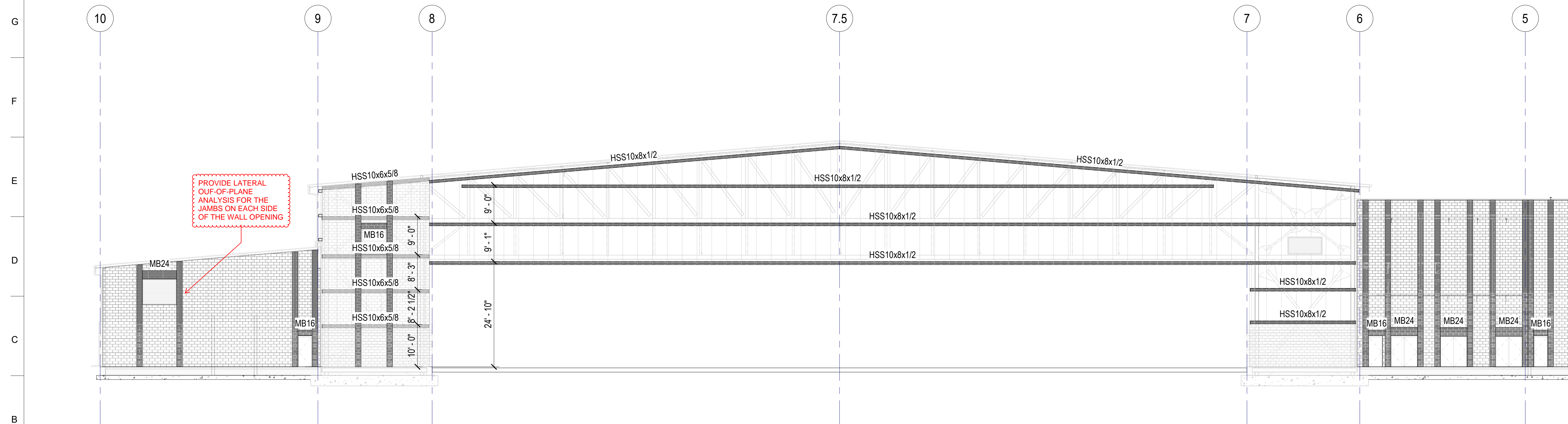
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DRAWN BY: R. CARLSON	SOLICITATION NO.: WB12PL23RA0012
CHECKED BY: D. CLAYSON	CONTRACT NO.: WB12PL25C0037
SUBMITTED BY: P. PASZCZUK	SIZE: ANSI D
US ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT	
KORTE CONSTRUCTION 5700 OAKLAND AVE, SUITE 275 ST. LOUIS, MO 63110	

CREECH AIR FORCE BASE, CLARK COUNTY, NV DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2 494.37	WIND GIRT ELEVATIONS
--	----------------------

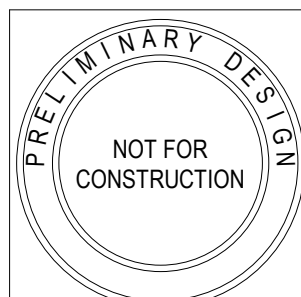
SHEET ID S-220



WIND GIRT ELEVATION - NORTH FACE, (WEST)



A2 WIND GIRT ELEVATION - NORTH FACE, (EAST)
1:150 REF: SB103

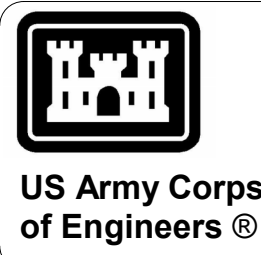
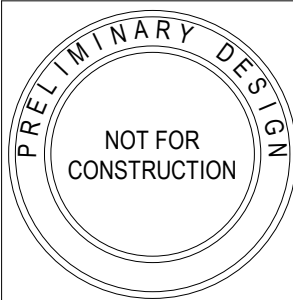
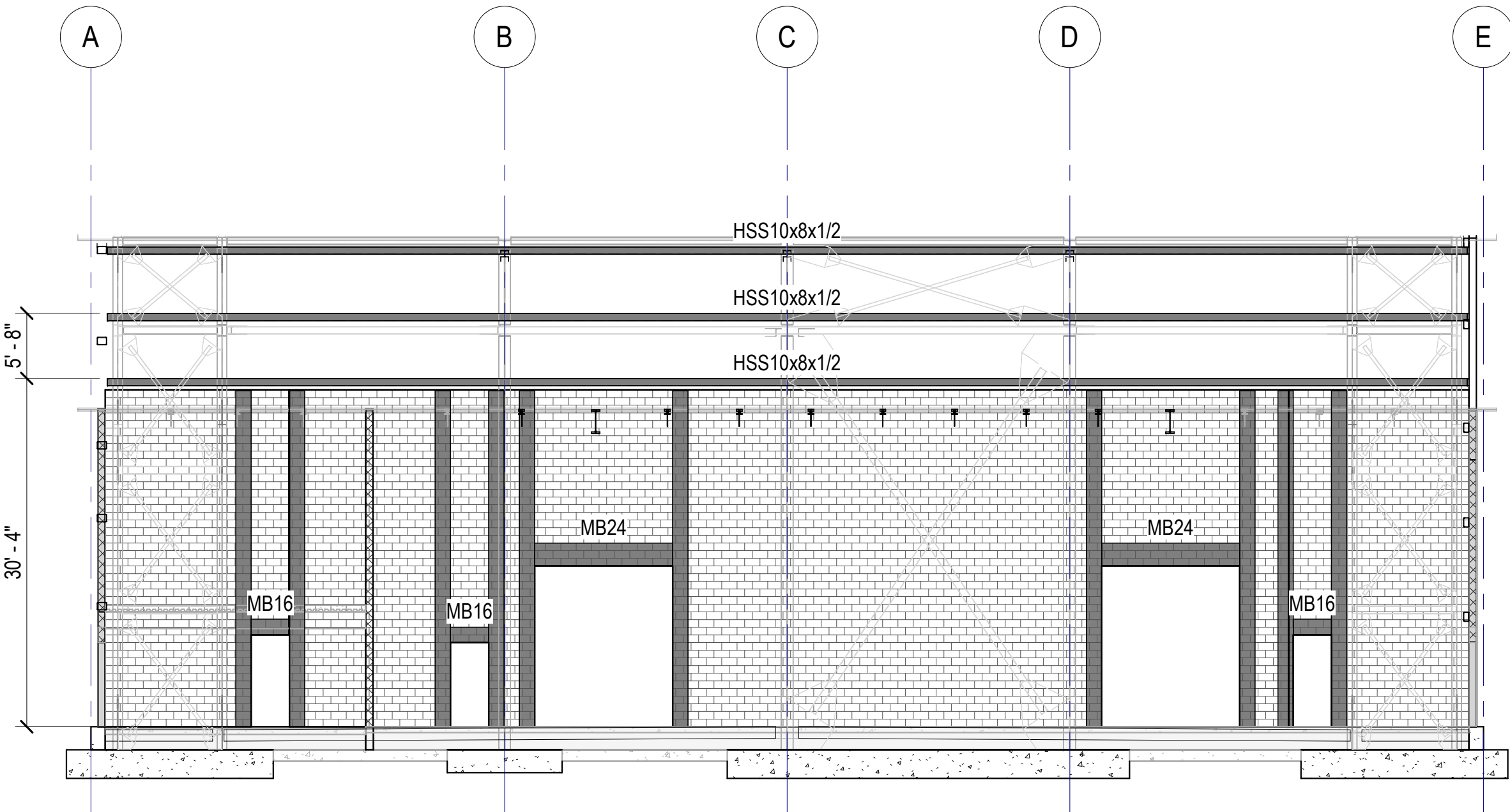
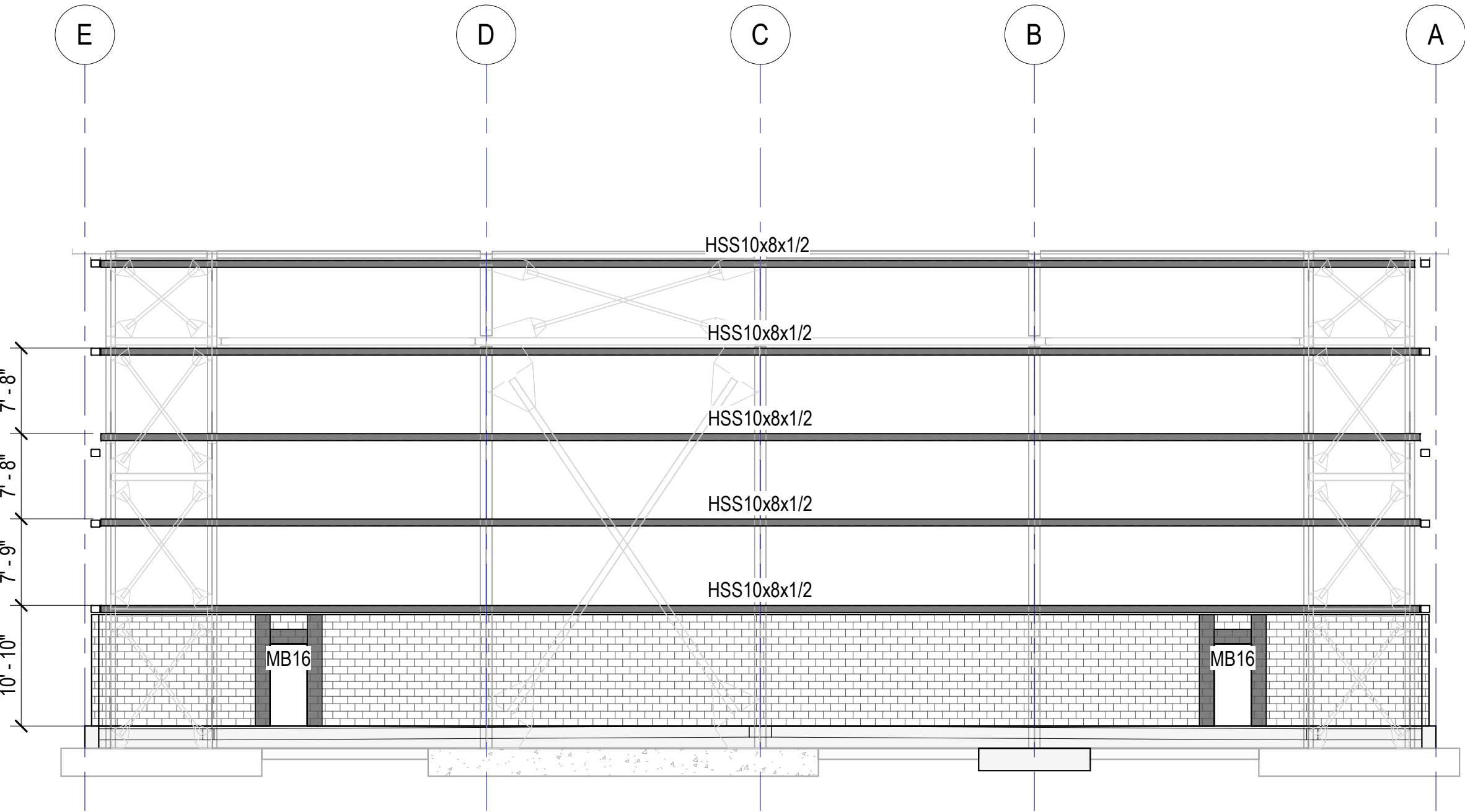
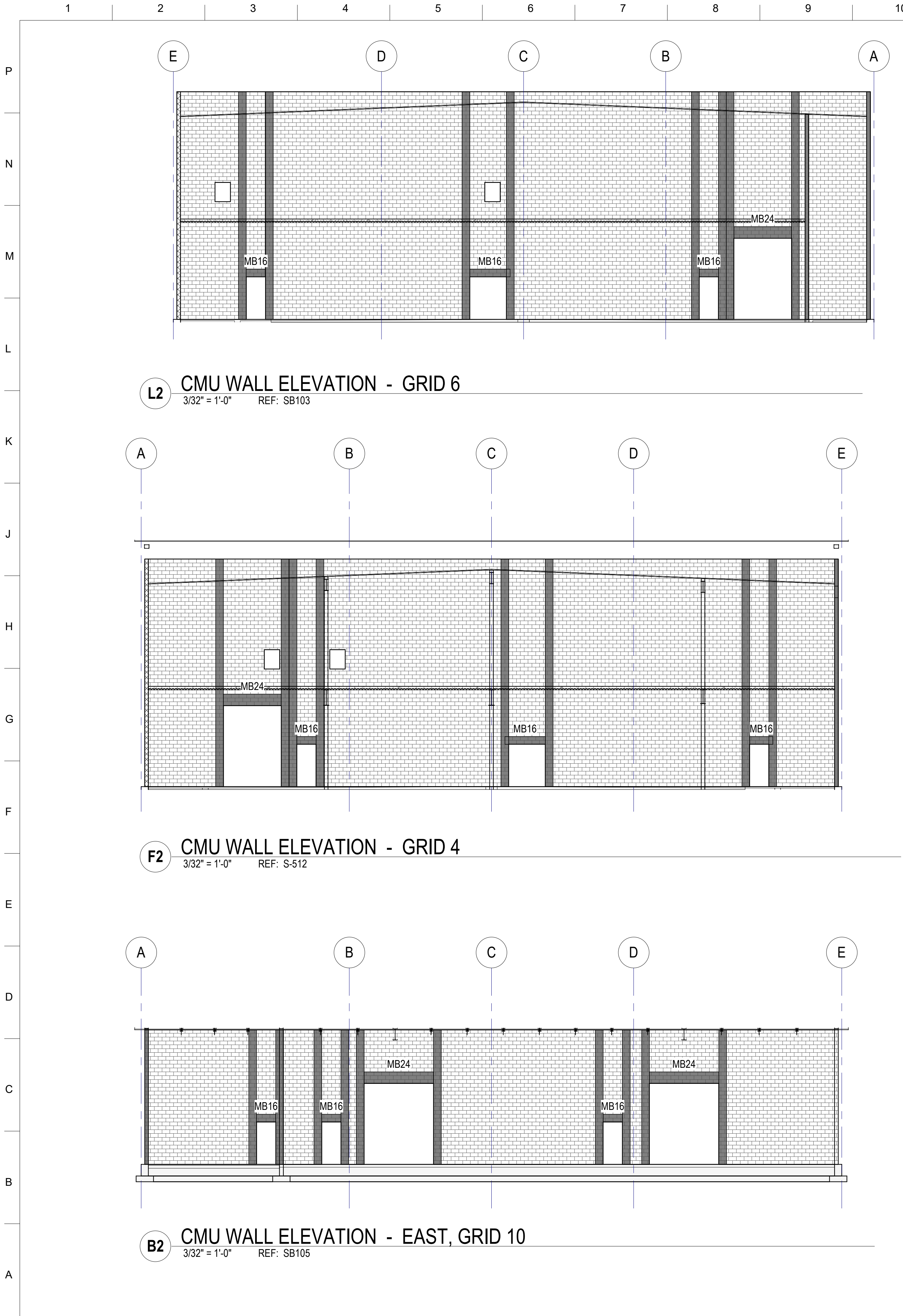
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US ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT	DESIGNED BY: A. VALENCIA	ISSUE DATE: NOVEMBER 13, 2025
	DRAWN BY: R. CARLSON	SOLICITATION NO.: W912PL259RA0012
KORTE CONSTRUCTION 5700 OAKLAND AVE. SUITE 275 ST. LOUIS, MO 63110	CHECKED BY: D. CLAYSON	CONTRACT NO.: W912PL25C00037
	SUBMITTED BY: P. PASZCZUK	
	SIZE: ANSI D	

GREECH AIR FORCE BASE, CLARK COUNTY, NV
DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2
494137

SHEET ID

S-221

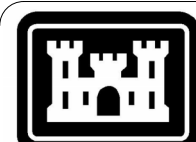
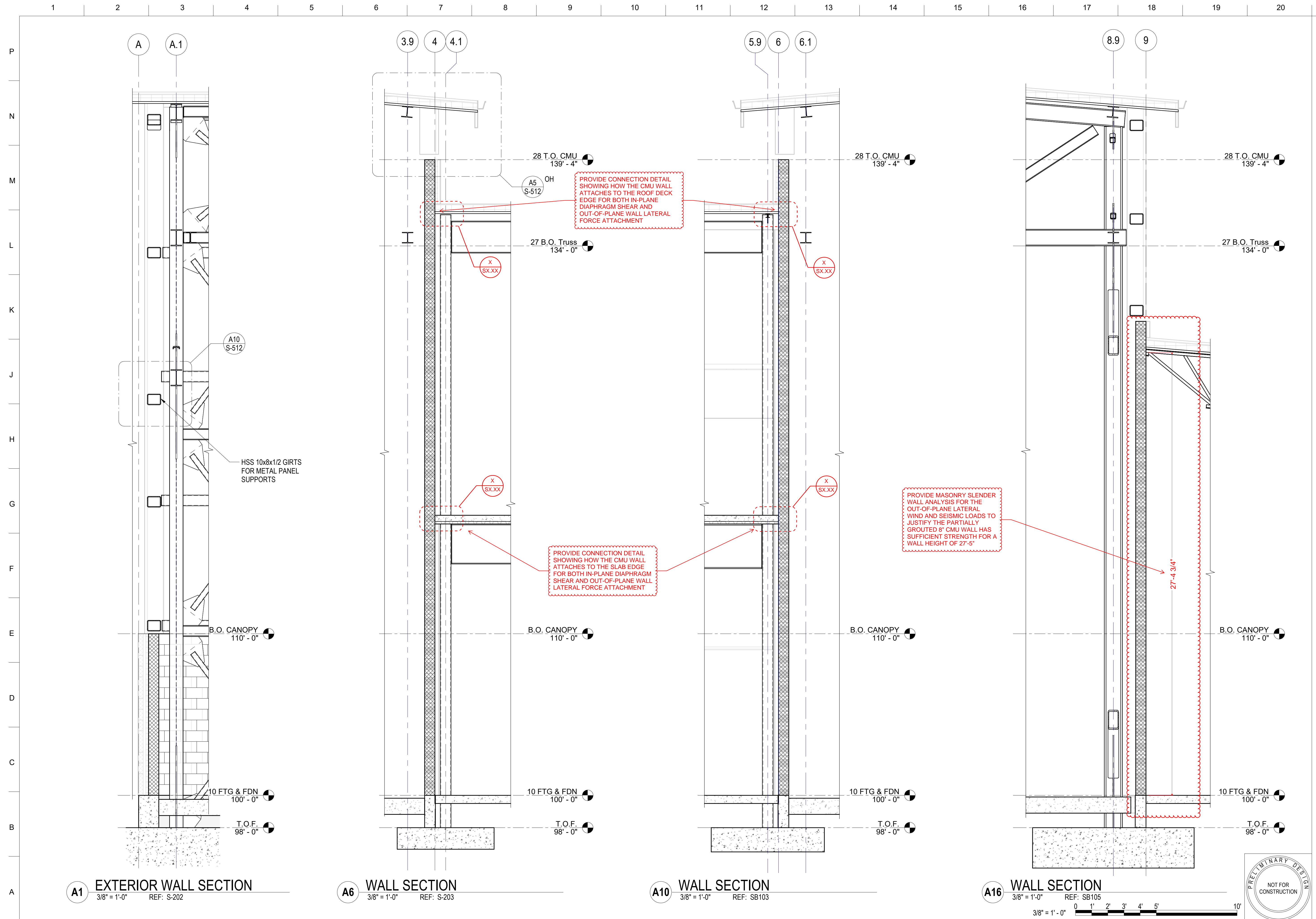


MARK	DESCRIPTION	DATE

DESIGNED BY: A. VALENCIA	ISSUE DATE: NOVEMBER 13, 2025
DRAWN BY: R. CARLSON	SOLICITATION NO.: WB12PL23RA0012
CHECKED BY: D. CLAYSON	CONTRACT NO.: WB12PL25C0037
SUBMITTED BY: P. PASZCZUK	SIZE: ANSI D
US ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT	
KORTE CONSTRUCTION 5700 OAKLAND AVE, SUITE 275 ST. LOUIS, MO 63110	

CREECH AIR FORCE BASE, CLARK COUNTY, NV DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2 494.37	WIND GIRT / CMU WALL ELEVATIONS
--	---------------------------------

SHEET ID S-222



**US Army Corps
of Engineers ®**

[illegible]

US ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT	DESIGNED BY: A. VALENCIA	ISSUE DATE: NOVEMBER 13, 2025
KORTE CONSTRUCTION 5700 OAKLAND AVE. SUITE 275 ST. LOUIS, MO 63110	DRAWN BY: D. CLAYTON	SOLICITATION NO.: W912PL25C0037
	CHECKED BY: D. CLAYTON	CONTRACT NO.: W912PL25C0037
	SUBMITTED BY: D. CLAYTON	ANSI D

CREECH AIR FORCE BASE, CLARK COUNTY, NV
DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2
494137
WALL SECTIONS

SHEET ID

S-301

DP-1 95% SUBMISSION

A B C D E F G H J K L M N P


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LOS ANGELES DISTRICT	A. VALENZUELA NOVEMBER 13, 2025 "NOV 13 2025" R. CARLSON W912PL25FA0012
KORTE CONSTRUCTION, 5700 OAKLAND AVE, SUITE 275 ST. LOUIS, MO 63110	CHECKED BY: D. CLAYSON CONTRACT NO.: W912PL2500037
	SUBMITTED BY: P. PASZCZUK DATE: ANS: D

494137

FOUNDATION DETAILS

S-502





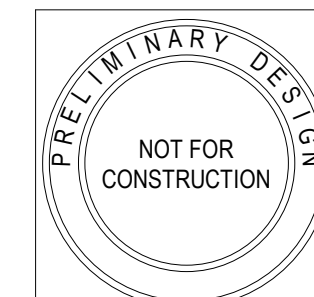
US ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT	DESIGNED BY: A. VALENCIA	ISSUE DATE: NOVEMBER 13, 2025
	DRAWN BY: R. CARLSON	SOLICITATION NO.: W912PL25RA0012
	CHECKED BY: D. CLAY SON	CONTRACT NO.: W912PL23C0037
KORTE CONSTRUCTION 5700 OAKLAND AVE. SUITE 275 ST. LOUIS, MO 63110	QUANTITY BY: P. KASZCZUN	
	ANSI D	

SHEET ID

S-512



CONNECTION DETAILS IN PROGRESS



This technical drawing illustrates a cross-section of a roof edge detail. It shows a parapet wall on the left, a horizontal beam or ledge in the center, and a vertical support structure on the right. A rectangular box highlights a specific area on the wall, likely indicating a point of interest or a detail to be further explored. The drawing is a line drawing with no shading.

[illegible][illegible]

NOTES:

1. DISCONTINUITIES RESULTING FROM FABRICATION AND ERECTION PROCEDURES AND FROM OTHER ATTACHMENTS ARE PROHIBITED IN THE REGION OF A MEMBER OR A CONNECTION ELEMENT DESIGNATED AS A PROTECTED ZONE.
2. WELDED STEEL HEADED STUD ANCHORS AND OTHER CONNECTIONS ARE PERMITTED IN PROTECTED ZONES.
3. WITHIN THE PROTECTED ZONE, HOLES, TACK WELDS, ERECTION AIDS, AIR-ARC GOUGING, AND UNSPECIFIED THERMAL CUTTING FROM FABRICATION OR ERECTION OPERATIONS SHALL BE REPAIRED AS REQUIRED BY THE ENGINEER OF RECORD (EOR).
4. STEEL HEADED STUD ANCHORS SHALL NOT BE PLACED ON BEAM FLANGE WITHIN THE PROTECTED ZONE.
5. ARC SPOT WELDS AS REQUIRED TO ATTACH DECKING ARE PERMITTED.
6. DECKING ATTACHMENTS THAT PENETRATE THE BEAM FLANGE SHALL NOT BE PLACED ON BEAM FLANGES WITHIN THE PROTECTED ZONE, EXCEPT POWER-ACTUATED FASTENERS UP TO 0.18 in (4.6 mm) DIAMETER ARE PERMITTED.
7. WELDED, BOLTED, OR SCREWED ATTACHMENTS OR POWER-ACTUATED FASTENERS FOR PERIMETER EDGE ANGLES, EXTERIOR FACADES, PARTITIONS, DUCT WORK, PIPING, OR OTHER CONSTRUCTION SHALL NOT BE PLACES WITHIN THE PROTECTED ZONE. EXCEPTION: OTHER ATTACHMENTS ARE PERMITTED WHERE DESIGNATED OR APPROVED BY THE EOR.
8. PROTECTED ZONES BE PERMANENTLY MARKED BY THE FABRICATOR AND RE-MARKED BY THE OWNER'S DESIGNATED REPRESENTATIVE IF THOSE MARKINGS ARE OBFUSCATED IN THE FIELD, SUCH AS BY APPLICATION OF FIREPROOFING.

COVER PLATE EA SIDE
CENTERED ON END OF
BRACE SLOT, SAME SIZE
AND WELD AS BRACE TO
GUSSET CONN, RE:
GUSSET PL SCHED

STEEL BRACE

1/4"

3/4" MAX

1" TYP

RE: S-602 GUSSET PL
SCHEDULE FOR W1 AND L1

12" WIDE (20" WIDE FOR
HSS8x8x5/8 BRACES) x 1/4"
THICK GUSSET PL SLOTTED
THROUGH CONTINUOUS
BRACE (NO WELD)

W1 L1 TYP

Diagram illustrating the elevation view of a steel column-to-deck connection. The components and labels are:

- STEEL ROOF DECK
- 10 GA CROWN PL
- 1/2" STIFFENER PL
- STEEL JOIST SEAT
- WORKPOINT, TYP
- TYPICAL CONNECTION (WELD AS PER DRAG STRUT DETAILS IN SCHEDULE)
- GUSSET PL AND WELD CONNECTION, RE: SCHEDULE
- 1/2" STIFFENER PL
- STEEL COLUMN, RE: PLAN

**US Army Corps
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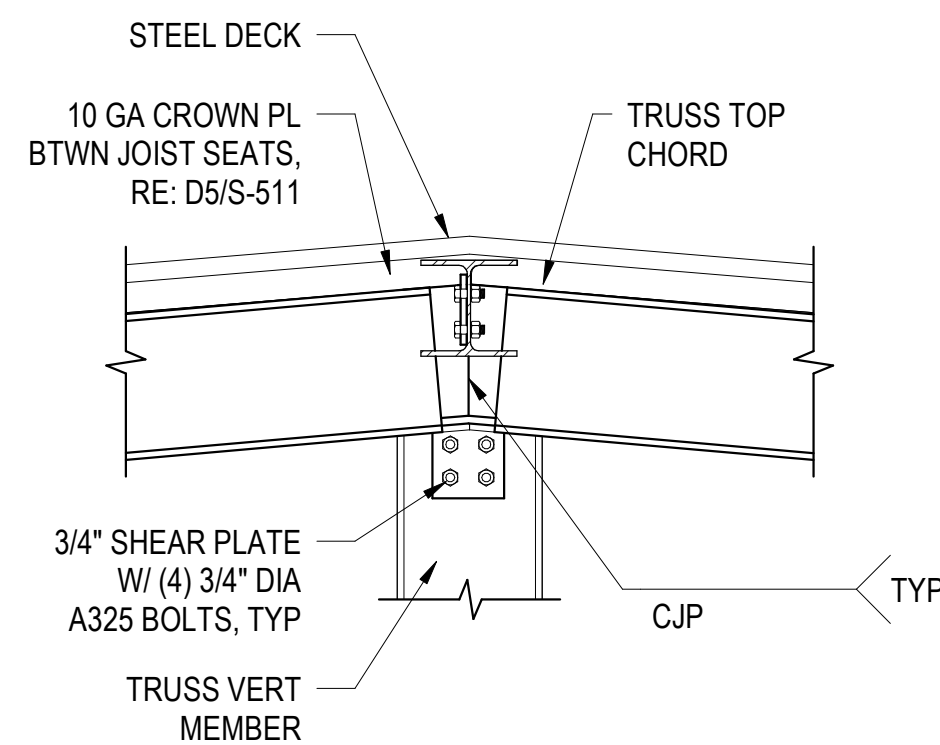
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US ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT	DESIGNED BY: A. VALENZUELA	ISSUE DATE: NOVEMBER 13, 2025
	DRAWN BY: R. CARLSON	SOLICITATION NO.: W912PL25FA0012
KORTE CONSTRUCTION 5700 OAKLAND AVE. SUITE 275 ST. LOUIS, MO 63110	CHECKED BY: D. CLAYSON	CONTRACT NO.: W912PL25C0037
	SUBMITTED BY: P. PASZCZYK	
	SIZE: ANSI D	

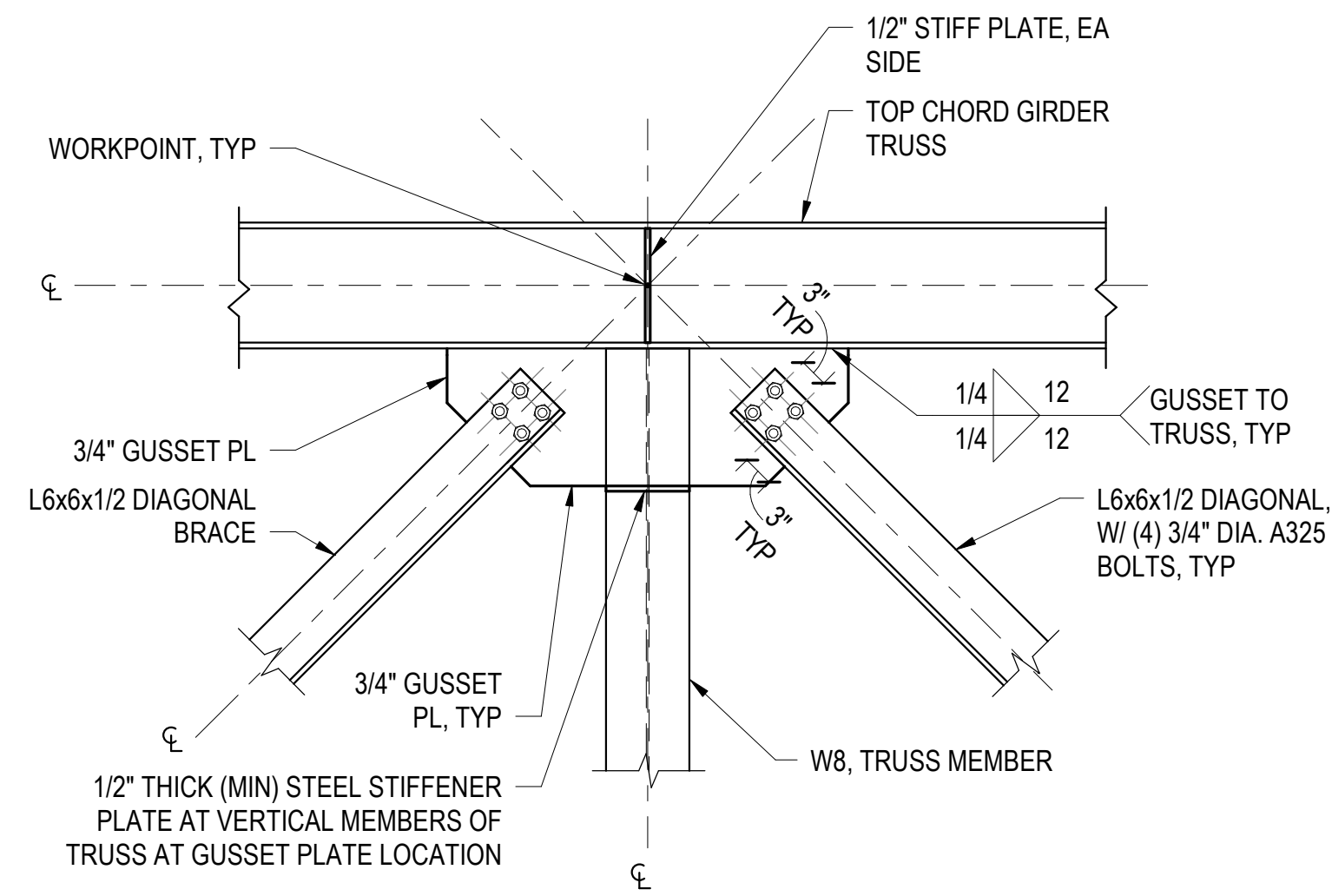
CREECH AIR FORCE BASE, CLARK COUNTY, NV
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494137

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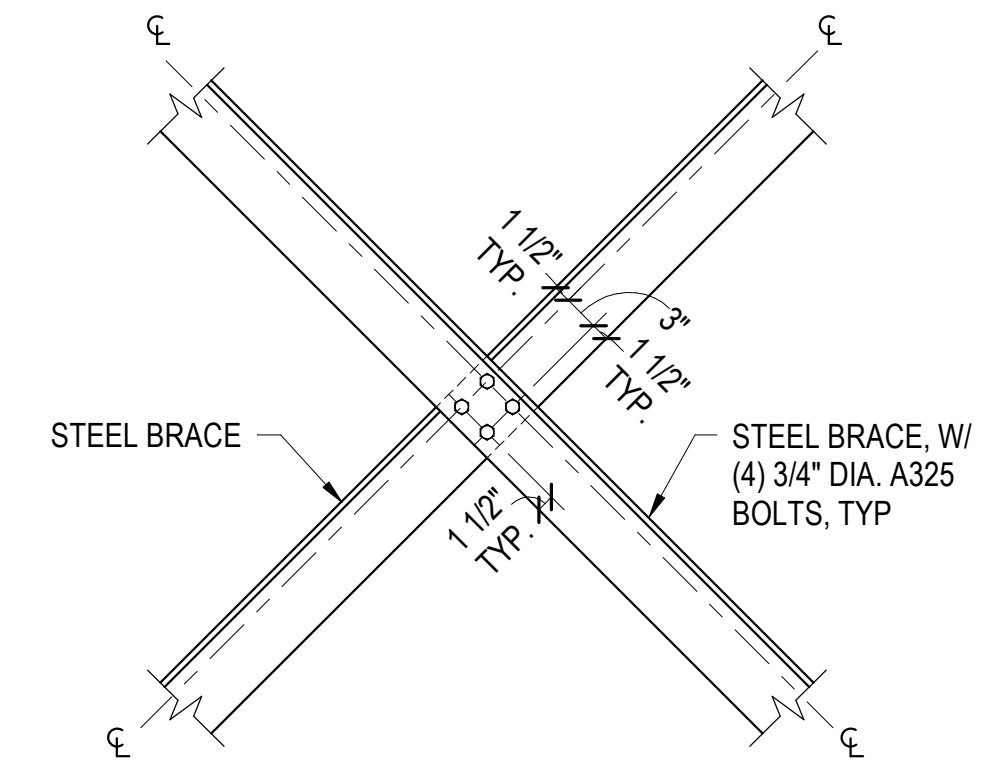
S-521



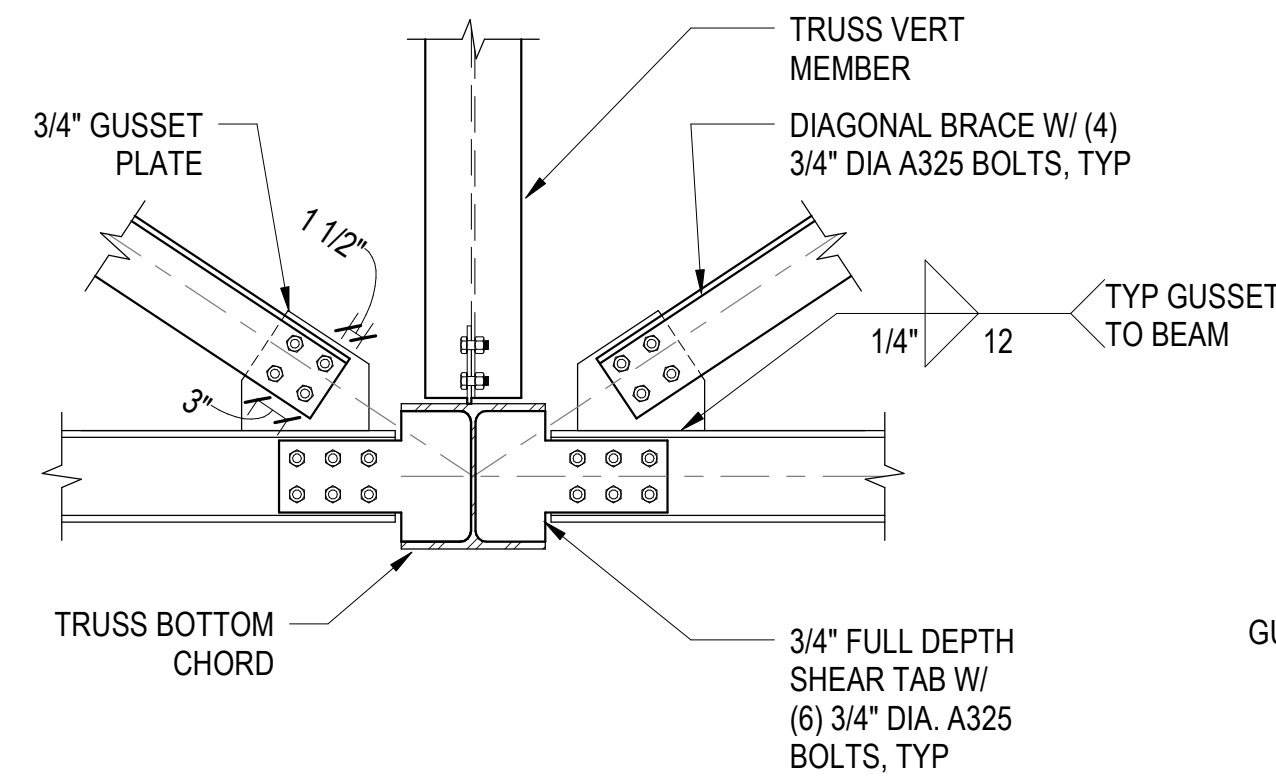
L6 TRUSS RIDGE
3/4" = 1'-0" REF: S-21



L10 **BRACE CONNECTION**
NTS REF: S-213

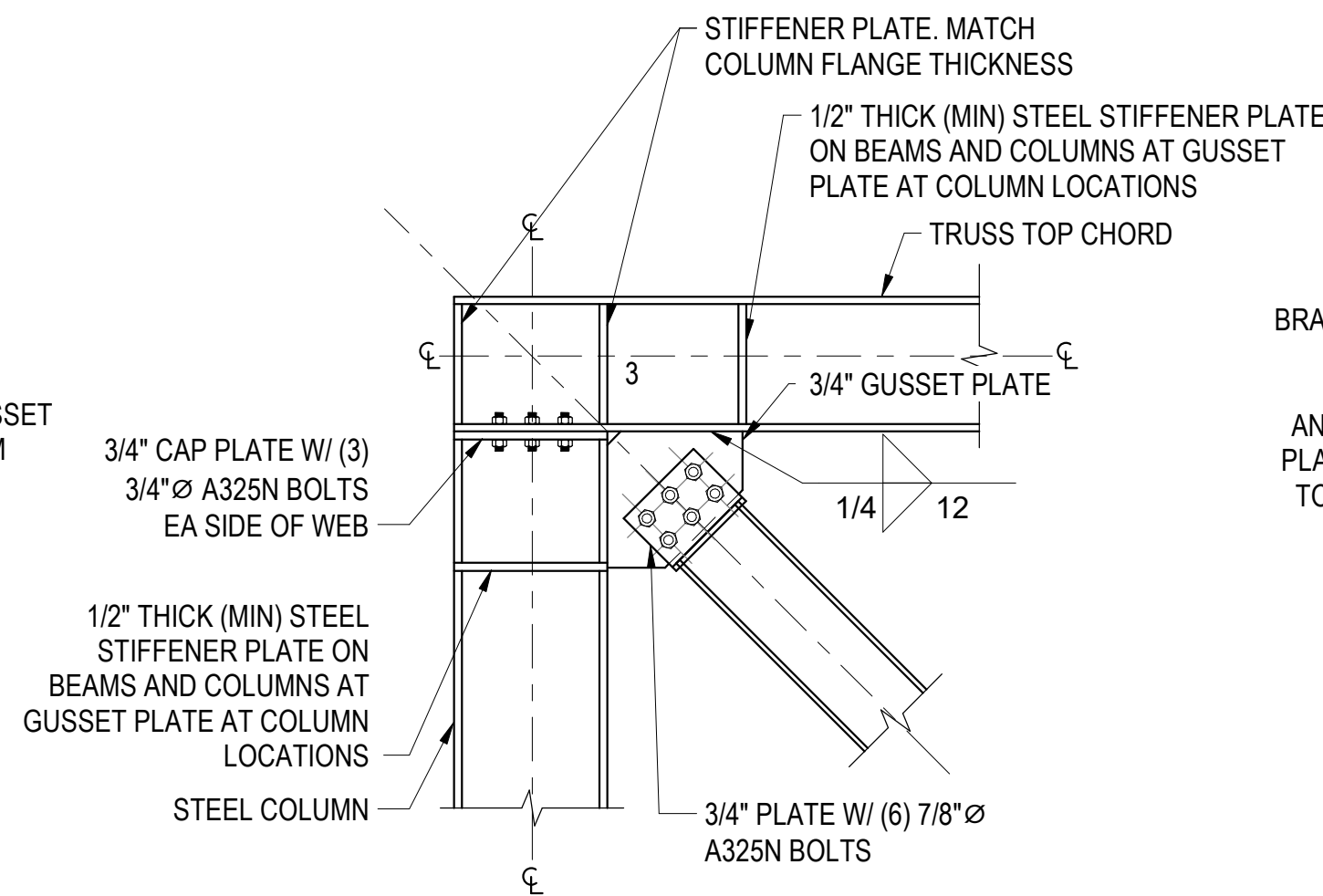


CROSS BRACING AT GIRDER TRUSS

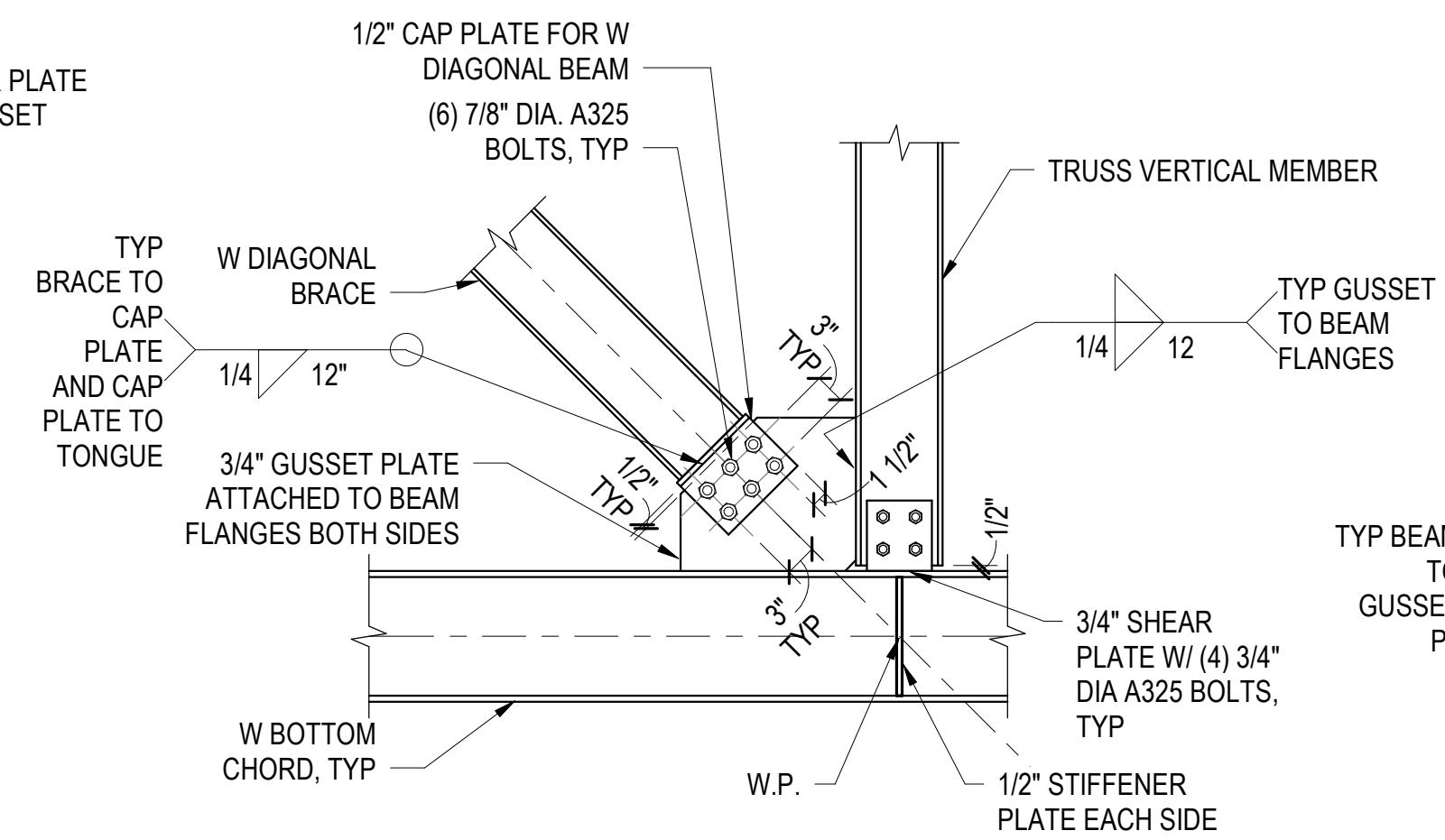


DIAGONAL BRACE ATTACHED TO
TRUSS BOT CHORD

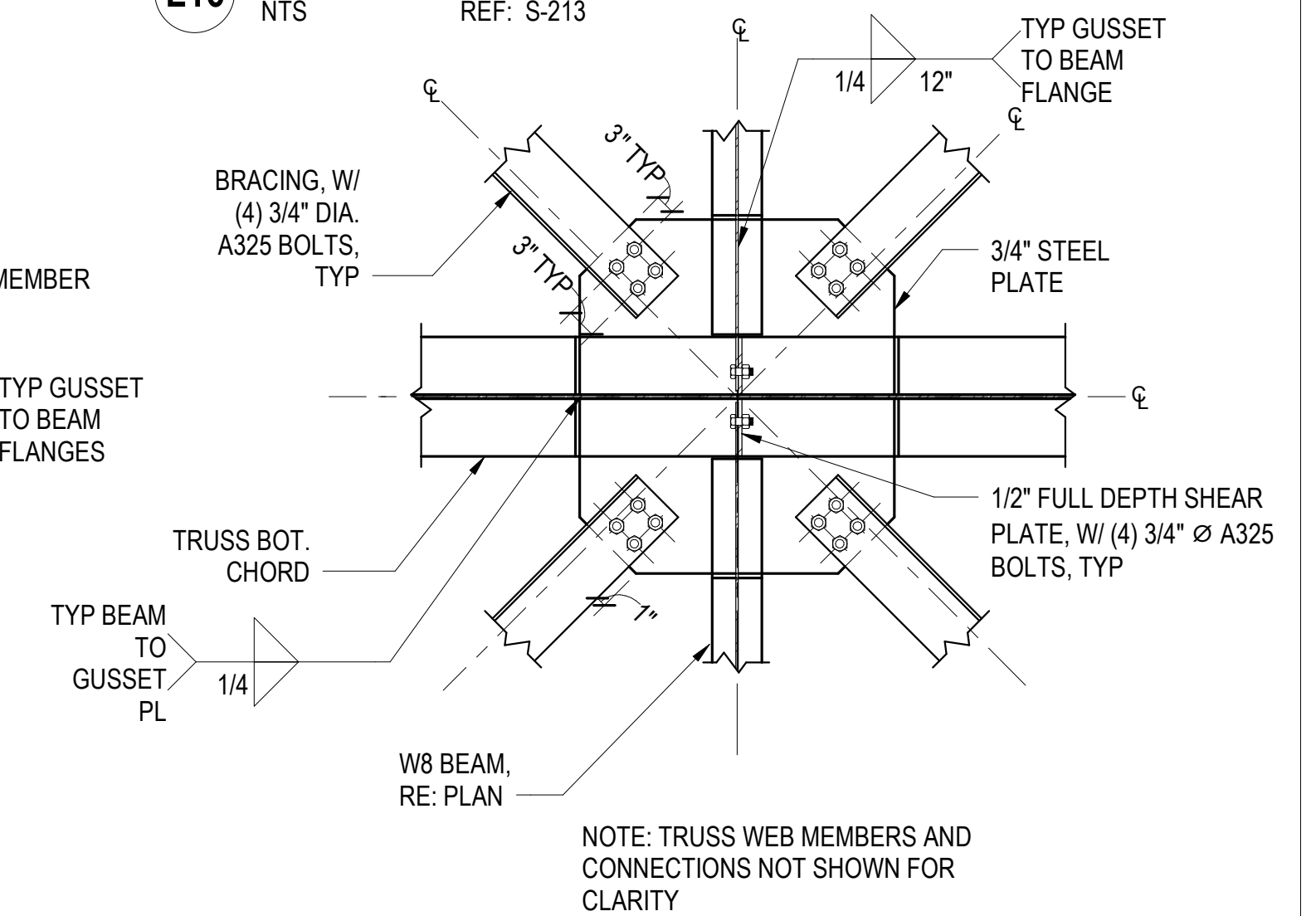
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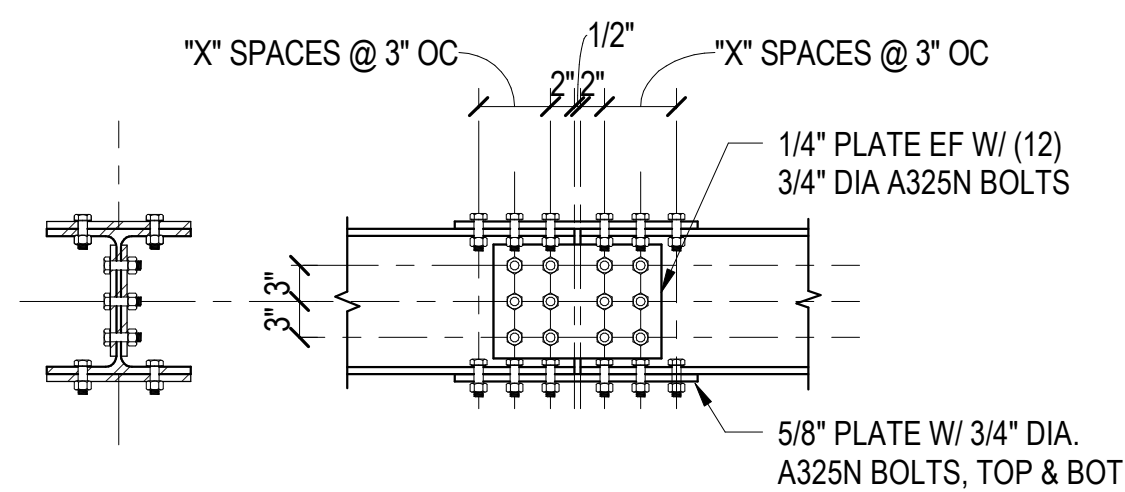
F6 TRUSS TOP CHORD AT STEEL COLUMN
NTS REF: S-211



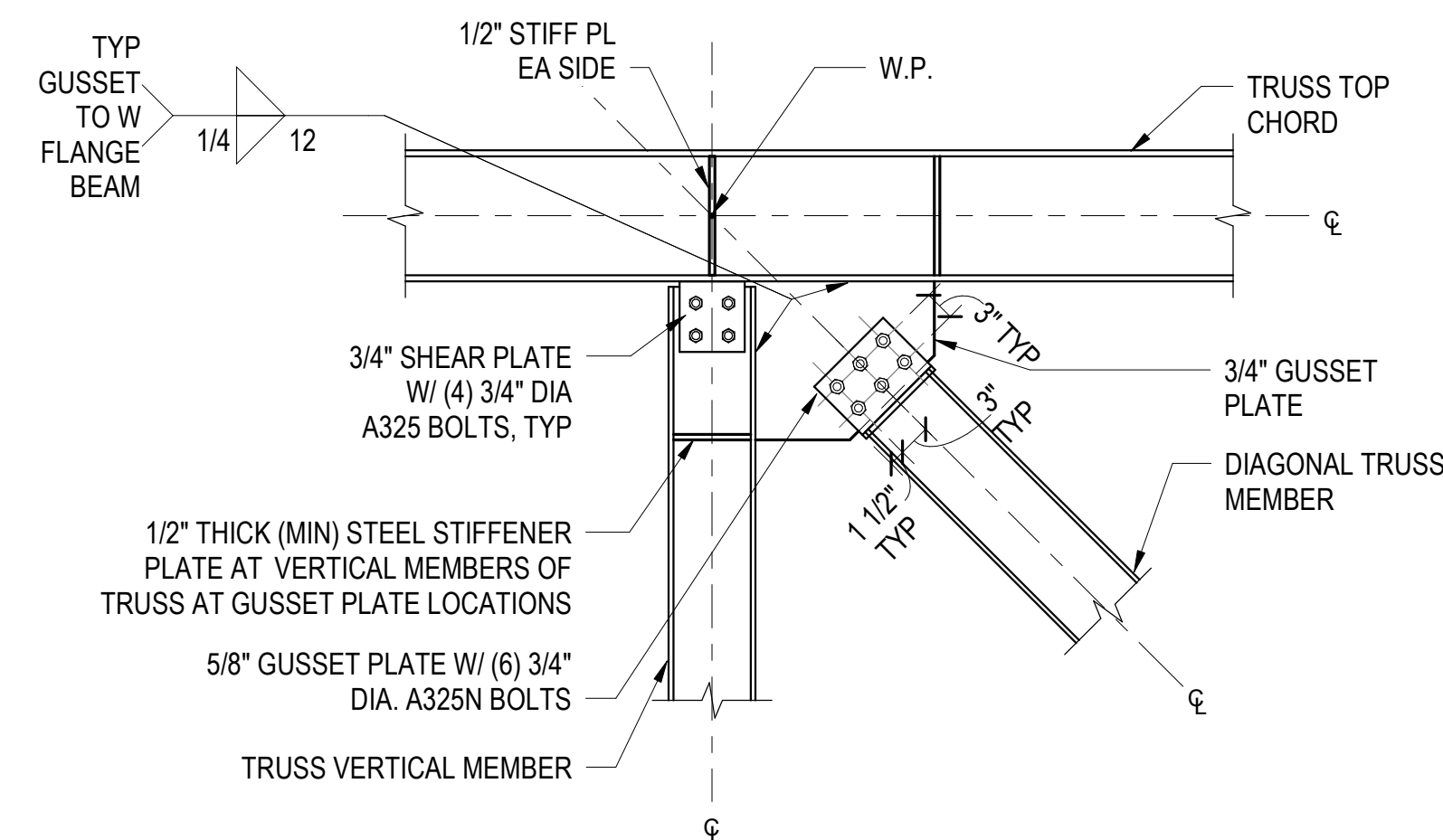
F10 TRUSS BOTTOM CHORD PANEL POINT
NTS REF: S-211



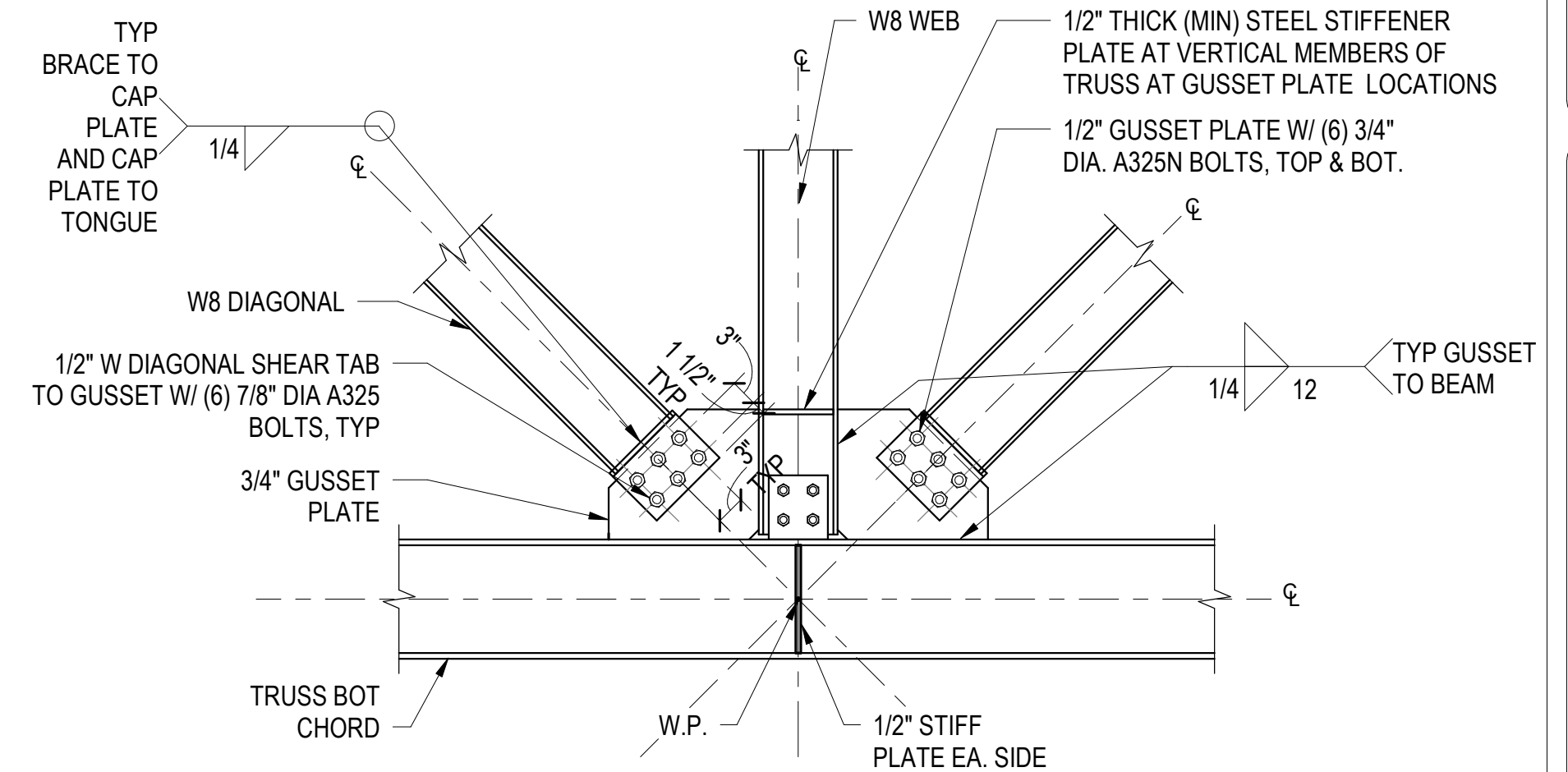
**PLAN - TYPICAL HORIZONTAL
BOTTOM CHORD BRACING**



A1 TRUSS CHORD SPLICE DETAIL
NTS



A9 TRUSS TOP CHORD PANEL POINT DETAIL
NTS



TRUSS BOTTOM CHORD PANEL
POINT DETAIL



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f Engineers ®**

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DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2

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TRUSS CONNECTION DETAILS

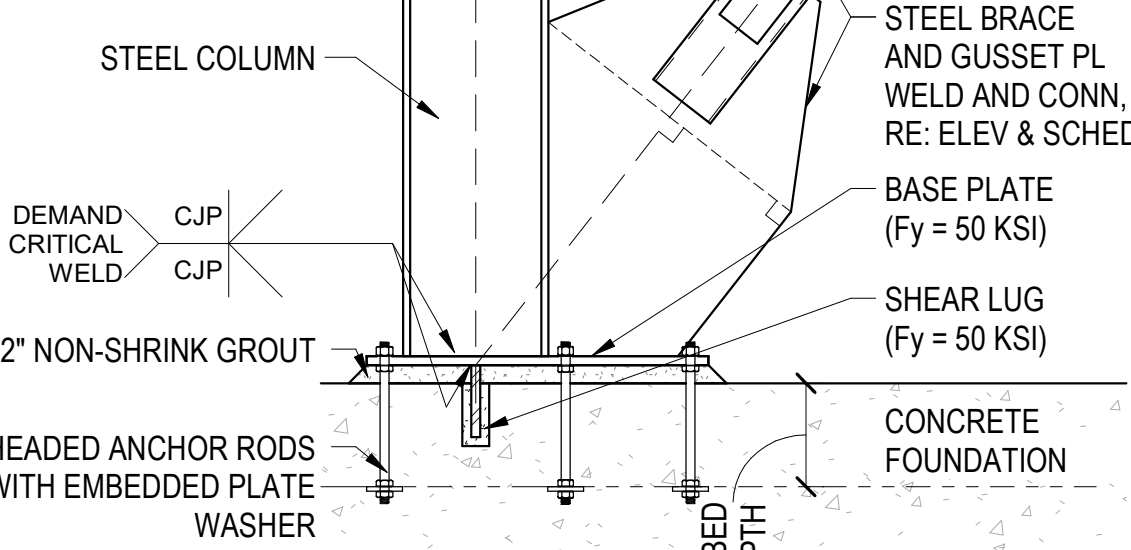
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S-531

FOOTING SCHEDULE

MARK	WIDTH	LENGTH	THICK	TRANSVERSE REINFORCING		LONGITUDINAL REINFORCING		NOTES
				NO.	SIZE	NO.	SIZE	
FS3.0	3' - 0"	3' - 0"	1' - 0"	(4)	#5	(4)	#5	
FS4.0	4' - 0"	4' - 0"	1' - 0"	(5)	#5	(5)	#5	
FS6.0	6' - 0"	6' - 0"	1' - 4"	(7)	#6	(7)	#6	
FS7.0	7' - 0"	7' - 0"	1' - 6"	(8)	#6	(8)	#6	TOP & BOTTOM
FS8x12	8' - 0"	12' - 0"	2' - 0"	(13)	#7	(9)	#7	TOP & BOTTOM
FS10.0	10' - 0"	10' - 0"	2' - 0"	(11)	#7	(11)	#7	TOP & BOTTOM
FS10x35	10' - 0"	35' - 0"	2' - 6"	(36)	#8	(11)	#8	TOP & BOTTOM
FS10x37	10' - 0"	37' - 0"	2' - 6"	(38)	#8	(11)	#8	TOP & BOTTOM
FS11.0	11' - 0"	11' - 0"	2' - 0"	(12)	#7	(12)	#7	TOP & BOTTOM
FS18x30	18' - 0"	30' - 0"	2' - 6"	(31)	#8	(19)	#8	TOP & BOTTOM
FC3.0	3' - 0"	CONT	1' - 0"	(4)	#5	—	#5 @ 12" OC	
FC4.0	4' - 0"	CONT	1' - 0"	(5)	#6	—	#5 @ 12" OC	

1. ALL FOOTINGS MUST BEAR ON PROPERLY PREPARED MATERIAL. SEE FOUNDATION SECTION OF THE STRUCTURAL GENERAL NOTES.
2. ALL FOOTINGS MUST BE CENTERED BELOW THE WALL AND/OR COLUMN ABOVE, TYP UNO.
3. ALL EARTH FORMED FOOTINGS MUST HAVE REQUIRED CONCRETE COVER FOR REINFORCEMENT PER THE CONCRETE COVER TABLE.
4. ALL EXTERIOR FOOTINGS MUST BEAR BELOW THE EFFECTS OF FROST. SEE THE DESIGN CRITERIA SECTION OF THE STRUCTURAL GENERAL NOTES FOR MINIMUM BEARING DEPTH.
5. PROVIDE MINIMUM COVER FOR ALL REINFORCING PER THE STRUCTURAL GENERAL NOTES AND/OR THE CONCRETE COVER SCHEDULE.
6. PLACE ALL FOOTING REINFORCING IN BOTTOM OF FOOTING WITH 3" CLEAR CONCRETE COVER, TYP UNO.
7. PLACE TRANSVERSE REINFORCING NEAREST EARTH AND LONGITUDINAL REINFORCING ON TOP OF TRANSVERSE REINFORCING.
8. PLACE TOP REINFORCING IF NOTED ON SCHEDULE. AS A MINIMUM, ALL FOOTINGS GREATER THAN OR EQUAL TO 18" IN THICKNESS REQUIRE #6 @ 12" OC EA WAY IN THE TOP OF FOOTING UNLESS THE SCHEDULE PROVIDES MORE STRINGENT REQUIREMENTS.
9. EXTEND CONTINUOUS FOOTINGS 12" MINIMUM PAST EDGE OF WALL, UNLESS OTHERWISE NOTED ON PLANS.
10. REINFORCING IN CONTINUOUS FOOTINGS MUST PASS THROUGH INTERSECTING SPOT FOOTINGS.
11. ALL REINFORCING FOR SPOT FOOTINGS AND MAT FOOTINGS AT BRACED FRAMES AND MOMENT FRAMES MUST HAVE A 90 DEGREE HOOK AT EA END.
12. PROVIDE DOWELS WITH STANDARD HOOKS FROM FOOTINGS TO ANY REINFORCED ELEMENT ABOVE WITH SIZE AND SPACING TO MATCH VERTICAL REINFORCING IN THE ELEMENT ABOVE.
13. ANY INCREASE IN THE SIZE OF FOOTINGS SHOWN MAY REQUIRE ADDITIONAL REINFORCING. COORDINATE WITH THE ENGINEER OF RECORD.
14. PENETRATIONS THROUGH FOOTINGS ARE NOT ALLOWED WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER OF RECORD.
15. ALL CONTINUOUS FOOTINGS MUST BE FC2.0 MINIMUM, AND ALL SPOT FOOTINGS MUST BE FS3.0 MINIMUM UNO ON PLANS.
16. SEE THE STRUCTURAL GENERAL NOTES FOR ADDITIONAL INFORMATION.



TYPICAL BRACED FRAME COLUMN BASE CONDITION

1. ALL BASE PLATES AND SHEAR LUGS MUST BE ASTM A572 GR50 STEEL, TYPE UNO
2. ALL ANCHOR RODS MUST BE ASTM F-1554 GR55 MIN UNO. THEY MUST BE HEADED ANCHOR RODS W/ 3"x3"x3/8" PLATE WASHERS WITH DOUBLE NUTS OR EMBED PLATE EMBEDDED IN CONCRETE AT THE EMBEDMENT DEPTH SPECIFIED, TYPE UNO.
 - A. ALL ANCHOR RODS MUST HAVE HARDENED WASHERS AND NUTS, WITH FULL HEIGHT OF EXTENSIONS THREADED
 - B. WASHERS MUST CONFORM TO AISC STEEL CONSTRUCTION MANUAL TABLE 14-2
 - C. BASE PLATE HOLES MAY INCREASE PER AISC STEEL CONSTRUCTION MANUAL TABLE 14-2
3. ALL BASE PLATES MUST BEAR ON MIN 1 1/2" THICK (2" THICK AT BRACED FRAMES) 5000 PSI NON-SHRINK GROUT AND MUST HAVE LEVELING NUTS, TYPE UNO
4. ALL BASE PLATES MUST BE WELDED TO THE COLUMN WITH A 1/4" FILLET WELD ALL AROUND, TYPE UNO
5. ALL ANCHOR RODS MUST BE SET IN PLACE WITH A TEMPLATE. THEY MUST BE PLACED PLUMB AND AT THE CORRECT DEPTH AND EXTENSION
6. THE WIDTH OF ALL SHEAR LUGS IS THE SAME AS THE 'N' DIMENSION SHOWN IN BASE PLATE TYPES
7. NOTCH SHEAR LUGS AS REQ'D TO ACCOMMODATE REINF STEEL
8. SEE THE STRUCTURAL GENERAL NOTES FOR ADDITIONAL INFORMATION



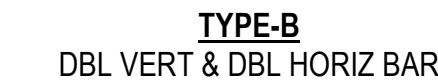
*PROVIDE EMBED PLATE, RE: K15/S-502



*PROVIDE EMBED PLATE, RE: K15/S-502

PROVIDE CALCULATIONS TO JUSTIFY THE PARTIALLY GROUTED 8" CMU WALLS WITH THIS REBAR LAYOUT WILL WORK FOR ALL 8" WALL CONDITIONS (1-STORY AND 2-STORY) FOR AXIAL, IN-PLANE SHEAR WALL AND LATERAL OUT-OF-PLANE WALL LOADS.

1. SEE GENERAL DETAILS FOR REINFORCING AT CORNERS, INTERSECTIONS, AND OPENINGS.
2. GROUT ALL CELLS SOLID THAT CONTAIN REINFORCING, EMBEDS, AND/OR BOLTS, TYP.
3. DO NOT SOLID GROUT WALLS UNO.
4. ALL MASONRY BELOW GRADE MUST BE GROUTED SOLID.
5. LAY ALL BLOCK IN RUNNING BOND, TYP UNO.
6. HORIZONTAL WALL REINF MUST CONTINUE THROUGH LINTELS. WHERE BOTH HORIZ WALL AND LINTEL REINF OCCUR IN THE SAME COURSE, USE THE LARGER REINFORCEMENT ONLY.
7. ALL HORIZ REINF MUST TERMINATE AT ENDS OF WALL AND JAMBS WITH STANDARD 180 DEG HOOKS. PLACE ADDITIONAL VERT BAR IN CENTER OF WALL IF NECESSARY.
8. PROVIDE SCHEDULED BOUNDARY COLUMNS AT END OF WALLS. SEE TYP MASONRY ELEVATION.
9. AT TOP AND BOTTOM OF WALL PROVIDE (2) #5 CONT IN ADDITION TO SCHEDULED REINFORCING.
10. AT ALL DECK AND JOIST EMBED LOCATIONS, PROVIDE (2) #5 CONT IN ADDITION TO SCHEDULED REINFORCING.
11. PROVIDE DOWELS WITH STANDARD HOOKS AND/OR PROPER LAP LENGTH TO THE STRUCTURE ABOVE AND BELOW WITH SIZE AND SPACING TO MATCH THE VERT REINF IN THE WALL, TYP UNO.
12. THE LAP SPlice LENGTH OF VERT REINF MUST BE AS SHOWN IN THE MASONRY REINF LAP SPlice TABLE IN THE GENERAL NOTES. ADJUST HEIGHT OF EACH LIFT AS REQUIRED.
13. WHEN A SINGLE CURTAIN OF REINF IS SPECIFIED, PLACE THE VERT REINF IN THE CENTER OF THE WALL, TYP UNO.
14. WHEN A DOUBLE CURTAIN OF REINF IS SPECIFIED, PLACE EACH CURTAIN AT THE FACE OF THE WALL WITH THE VERT REINF CLOSEST TO THE SHELL WITH A CLEAR DISTANCE BETWEEN 1/2" AND 1" TO THE INSIDE FACE OF THE SHELL.
15. ALL WALLS MUST INCLUDE LADDER TYPE JOINT REINF SPACED AT 16" OC VERTICALLY WITH AT LEAST TWO WIRES OF W1.7 (GALVANIZED).
16. SEE GENERAL STRUCTURAL NOTES FOR ALL OTHER REQUIREMENTS.



MASONRY WALL TYPES

WHERE CONTROL JOINT OCCURS AT EDGE OF OPENINGS,
ALL WALL BEAM REINF AND GROUT MUST BE CONTINUOUS
THROUGH JOINT. USE OPEN END UNITS OR BREAK OUT TOP
HALF OF SHELL FOR GROUT FLOW @ JOINT OF BEAM.
DELETE PREFORMED GASKET FOR HEIGHT OF BEAM

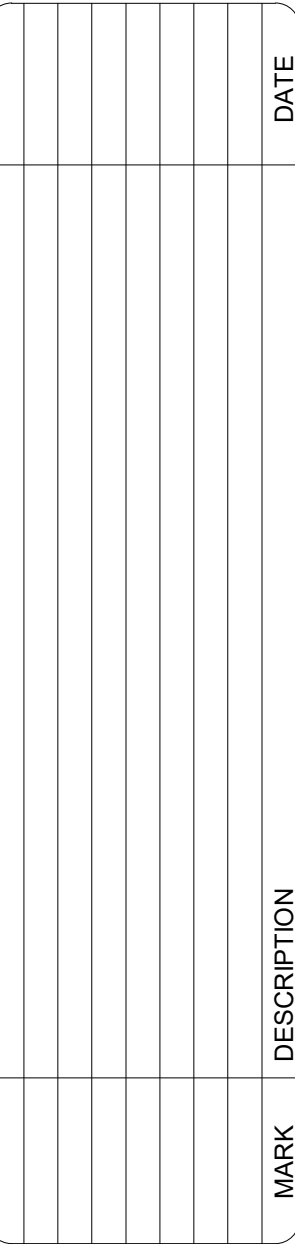
MASONRY WALL CONTROL JOINT



1. SEE TYPICAL DETAILS FOR REINFORCING AT CORNERS, INTERSECTIONS, AND OPENINGS.
2. PROVIDE DOWELS WITH STANDARD HOOKS AND/OR PROPER LAP LENGTH TO THE STRUCTURE ABOVE AND BELOW WITH SIZE AND SPACING TO MATCH THE VERT REINF IN THE WALL, TYP UNO.
3. THE LAP SPLICE LENGTH OF VERT REINF MUST BE AS SHOWN IN THE CONCRETE REINF DEVELOPMENT AND LAP SPLICE TABLE IN THE GENERAL NOTES. ADJUST HEIGHT OF EACH LIFT AS REQUIRED.
4. WHEN A SINGLE CURTAIN OF REINF IS SPECIFIED, PLACE THE VERT REINF IN THE CENTER OF THE WALL, TYP UNO.
5. AT TOP AND BTM OF WALL, INCLUDING ALL DECK BEARING ELEVATIONS, PROVIDE (2) #5 CONT IN ADDITION TO SCHEDULED REINFORCING.
6. ALL HORIZONTAL REINF MUST TERMINATE AT ENDS OF WALLS AND ALL JAMBS WITH A STANDARD 180 DEGREE HOOK. END OF WALL IS DEFINED AS ANY WALL SEGMENT THAT EITHER CHANGES DIRECTION AND/OR CHANGES TO A DIFFERENT WALL TYPE.
7. SEE GENERAL STRUCTURAL NOTES FOR ALL OTHER REQUIREMENTS.



REINFORCED CONCRETE WALL TYPES



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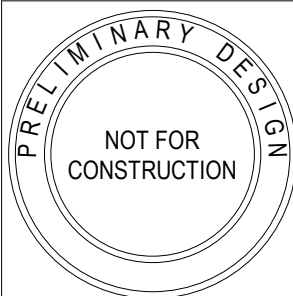
US ARMY CORPS OF ENGINEERS
LOS ANGELES DISTRICT

KORTE CONSTRUCTION
5700 OAKLAND AVE, SUITE 275
ST. LOUIS, MO 63110

CREECH AIR FORCE BASE, CLARK COUNTY, NV
DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2
494137

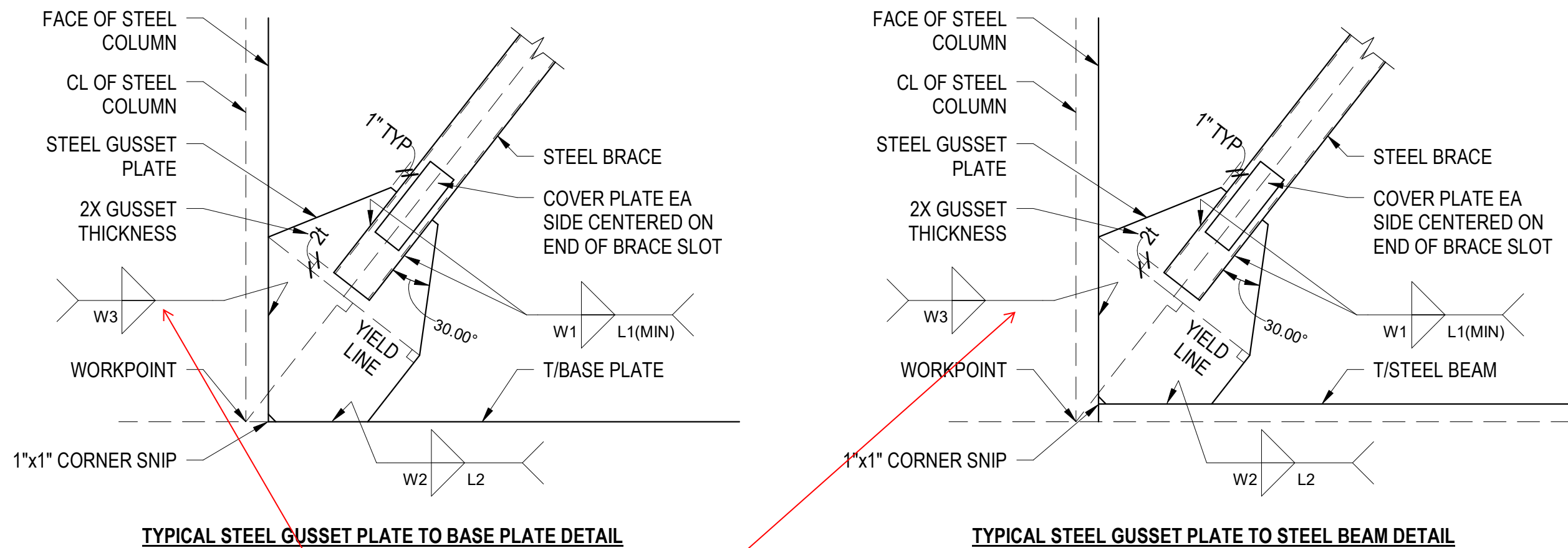
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S-601



STEEL GUSSET PLATE CONNECTION SCHEDULE

MARK	GUSSET PLATE THICKNESS	GEOMETRY AND WELDING INFORMATION					COVER PLATE INFO		NOTES
		W1 SIZE	L1 LENGTH	W2 SIZE	L2 LENGTH	W3 SIZE	PLATE SIZE	WELD	
1	1"	5/16	3' - 3"	7/16	3' - 1"	7/16	1/2"x4"x1'-0"	1/4	
2	1"	5/16	3' - 3"	7/16	2' - 8"	7/16	1/2"x4"x1'-0"	1/4	
3	7/8"	5/16	1' - 7"	5/16	2' - 10"	3/8	1/2"x4"x1'-0"	1/4	
4	7/8"	5/16	1' - 7"	5/16	2' - 5"	3/8	1/2"x4"x1'-0"	1/4	
5	7/8"	5/16	1' - 7"	5/16	2' - 5"	3/8	1/2"x4"x1'-0"	1/4	
6	7/8"	5/16	1' - 7"	5/16	3' - 1"	3/8	1/2"x4"x1'-0"	1/4	
7	7/8"	5/16	1' - 7"	5/16	2' - 3"	3/8	1/2"x4"x1'-0"	1/4	
8	7/8"	5/16	1' - 7"	5/16	2' - 8"	3/8	1/2"x4"x1'-0"	1/4	
9	7/8"	5/16	3' - 3"	7/16	3' - 1"	3/8	1/2"x5"x1'-0"	1/4	
10	7/8"	5/16	3' - 3"	7/16	2' - 10"	7/16	1/2"x5"x1'-0"	1/4	
11	3/4"	5/16	1' - 0"	1/4	3' - 2"	1/4	1/2"x5"x1'-0"	1/4	
12	3/4"	5/16	1' - 0"	1/4	4' - 3"	1/4	1/2"x5"x1'-0"	1/4	
13	3/4"	5/16	1' - 0"	3/8	1' - 5"	3/8	1/2"x5"x1'-0"	1/4	
14	3/4"	5/16	1' - 0"	5/16	1' - 1"	3/8	1/2"x5"x1'-0"	1/4	
15	3/4"	5/16	1' - 0"	5/16	1' - 8"	5/16	1/2"x5"x1'-0"	1/4	
16	3/4"	5/16	1' - 0"	5/16	1' - 10"	5/16	1/2"x5"x1'-0"	1/4	



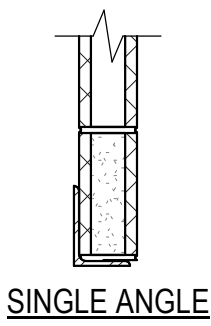
NOTES:

1. RE: BRACED FRAME ELEVATIONS FOR MARKED LOCATIONS OF EACH GUSSET ASSEMBLY.
2. ALL GUSSET PLATES MUST BE A572 GRADE 50 STEEL.
3. ALL COVER PLATES MUST BE A572 GRADE 50 STEEL.
4. AT CONTRACTOR'S OPTION, FILLET WELDS MAY BE REPLACED WITH CJP WELDS SO LONG AS REQUIRED TESTING IS PERFORMED PER GOVERNING BUILDING CODE.
5. YIELD LINE SHOULD EXACTLY INTERSECT WITH COLUMN FACE OR BASE PLATE/BASE FACE DEPENDING ON THE GEOMETRY.
6. LENGTH L1 IS A MINIMUM WELD LENGTH. USE LENGTH L2 AS A BASELINE TO ESTABLISH THE YIELD LINE.
7. PLACE 1/2" THICK FOAM EACH SIDE OF GUSSET PLATE WHEN CONCRETE POURS AROUND GUSSET PLATE.

ALSO IDENTIFY THE LENGTH OF WELD ALONG THE VERTICAL GUSSET PLATE INTERFACE. ONLY IDENTIFYING THE HORIZONTAL "L2" WELD LENGTH DOESN'T FULLY DEFINE THE GUSSET PLATE GEOMETRY REQUIREMENTS TO PROPERLY ESTABLISH THE 2T YIELD (FOLD) LINE.

VENEER LINTEL SCHEDULE

LOOSE LINTEL SCHEDULE FOR NON-LOAD BEARING MASONRY WALLS	
OPENING WIDTH	WALL THICKNESS
UP TO 4'-0"	4" WALL L4x3 1/2x5/16 LLV
4'-0" TO 6'-0"	L5x3 1/2x5/16 LLV
6'-0" TO 8'-0"	L6x3 1/2x5/16 LLV
8'-0" TO 12'-0"	L6x6x1/2
12'-0" TO 16'-0"	L8x6x5/8 LLV



NOTES:

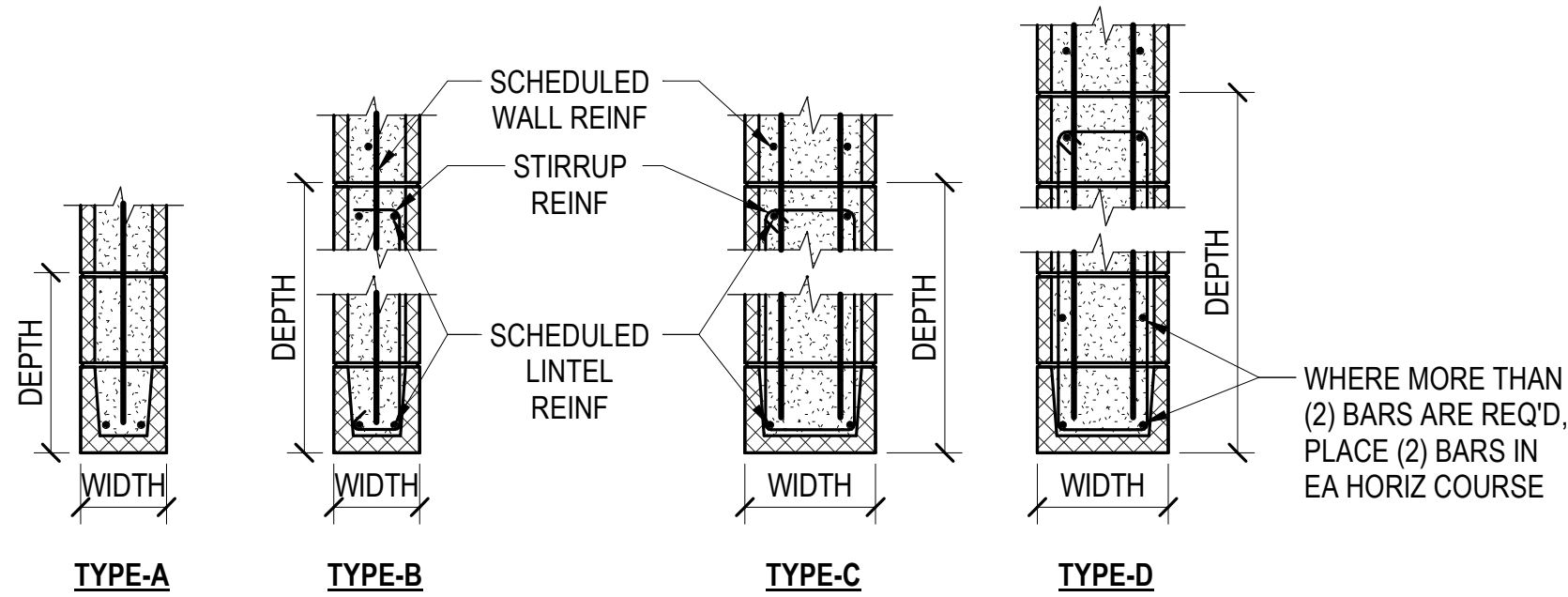
1. RE: STRUCTURAL, ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR OPENING SIZE AND LOCATION.
2. CONNECT ALL DOUBLE ANGLES BACK TO BACK AT 2'-0" OC MAXIMUM SPACING.
3. PROVIDE 6" MINIMUM BEARING AT FIRST FULL MASONRY CELL AT EACH END OF LOOSE LINTEL.
4. FOR OPENINGS 6'-0" AND WIDER, FULLY GROUT FIRST FULL CELL EACH SIDE OF OPENING FOR FULL HEIGHT OF WALL.
5. FOR OPENINGS LESS THAN 6'-0" WIDE, FULLY GROUT FIRST FULL CELL EACH SIDE OF OPENING FOR MINIMUM HEIGHT OF 8", BUT NOT LESS THAN THE FULL CELL HEIGHT, BELOW LINTEL BEARING ELEVATION.
6. FULLY GROUT ALL CELLS WHERE LOOSE LINTELS ARE LOCATED.
7. ANGLES IN EXTERIOR WALLS ARE TO BE GALVANIZED.

MASONRY LINTEL SCHEDULE

MARK	WIDTH	DEPTH	TYPE	LINTEL REINFORCING		NOTES
				HORIZONTAL	STIRRUPS	
MB16	7 5/8"	1' - 4"	A	(2) #5	N/A	
MB24	7 5/8"	2' - 0"	A	(2) #6 T+B	N/A	
MB32	7 5/8"	2' - 8"	A	(2) #5	N/A	

NOTES:

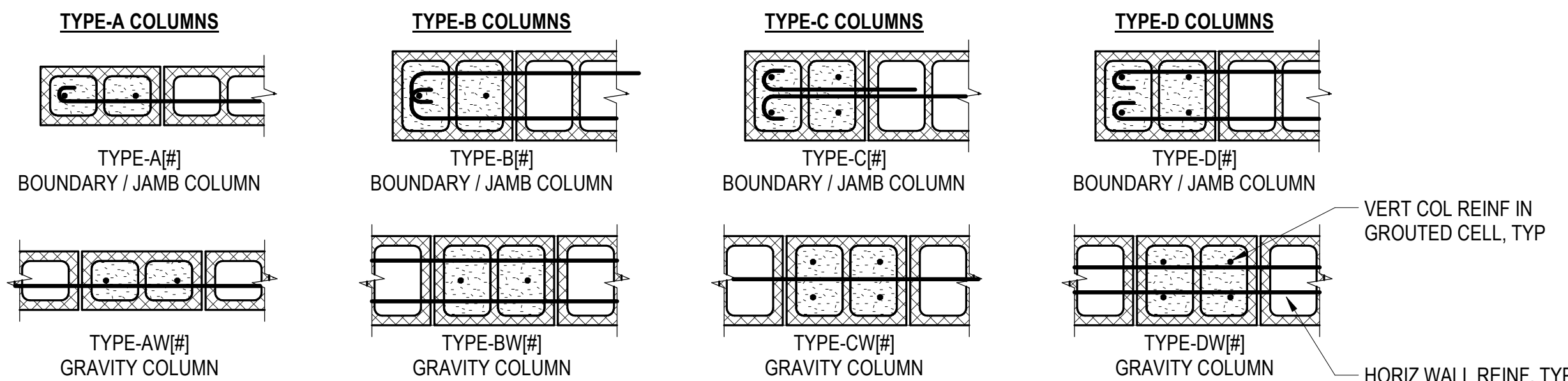
1. LINTELS MUST BE OF THE SAME MATERIAL AND WIDTH AS THE WALL IN WHICH THEY ARE CONSTRUCTED.
2. LINTELS MUST BE GROUTED MONOLITHICALLY WITH THE SUPPORTING WALL AND COLUMNS.
3. GROUT LINTELS SOLID FOR DEPTH SHOWN IN THE SCHEDULE, PLUS AS PER DETAILS, STRUCTURAL NOTES, AND/OR WALL SCHEDULE.
4. EXTEND HORIZONTAL REINFORCING 48 BAR DIAMETERS MIN BEYOND THE EDGE OF ALL OPENINGS. PROVIDE A 90° STANDARD HOOK WHERE THIS CANNOT BE ACCOMPLISHED.
5. NO DUCTS, OPENINGS, OR PENETRATIONS WILL OCCUR THROUGH BEAMS UNO.
6. REINFORCING INDICATED IN LINTEL SCHEDULE IS IN ADDITION TO WALL HORIZ AND VERT REINFORCING.
7. SEE GENERAL STRUCTURAL NOTES FOR ALL OTHER REQUIREMENTS.



MASONRY LINTEL TYPES

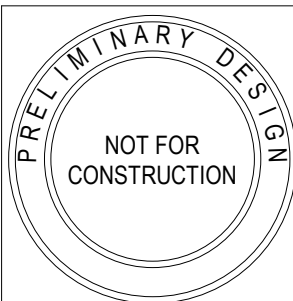
MASONRY JAMB AND COLUMN SCHEDULE

MARK	SIZE	TYPE	COLUMN REINFORCING		NOTES
			VERTICAL	TIES	
MP16	8x16	A2	#5 EA CELL	N/A	
MP32	8x32	A4	#6 EA CELL	N/A	



NOTES:

1. DESIGNATION "TYPE A#[]" WHERE "A" EQUALS THE WALL TYPE IN WHICH THE COLUMN/JAMB OCCURS AND WHERE [#] EQUALS THE NUMBER OF VERTICALLY GROUTED CELLS CONTAINING REINFORCING.
2. HORIZONTAL WALL REINF MUST RUN CONTINUOUS THROUGH MASONRY COLUMNS.
3. GROUT ALL REINFORCED CELLS AND VOIDS SOLID.
4. MASONRY COLUMN REINF MUST EXTEND FULL HEIGHT FROM MARK ON PLAN DOWN TO FOUNDATION AND TERMINATE WITH A STANDARD 90° HOOK. FOR CONC FOUNDATION WALLS HEIGHTS OVER 5'0". VERT MASONRY REINF MUST DOWEL 4'0" MIN INTO FOUNDATION WALL.
5. NUMBER OF VERT BARS IS TOTAL NUMBER OF BARS.
6. SEE ARCHITECTURAL DRAWINGS FOR SPECIAL COURSING ARRANGEMENTS.
7. ALL TIES MUST TERMINATE WITH A STANDARD MASONRY HOOK.
8. SEE MASONRY REINFORCING SPLICE LENGTH TABLES FOR REINF LAP LENGTHS.
9. SEE GENERAL STRUCTURAL NOTES FOR ALL OTHER REQUIREMENTS.

[illegible]

DESIGNED BY:	ISSUE DATE:
A. VALENCIA	NOVEMBER 13, 2025
DRAWN BY:	SOLICITATION NO.:
R. CARLSON	W912PL25RA0012
CHECKED BY:	CONTRACT NO.:
D. CLAYSON	W912PL25C00037
SUBMITTED BY:	
P. PASZCZUK	
SIZE:	
ANSI D	

US ARMY CORPS OF ENGINEERS
LOS ANGELES DISTRICT

KORTE CONSTRUCTION
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CREECH AIR FORCE BASE, CLARK COUNTY, NV
DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2

404137

STRUCTURAL SCHEDULES

SHEET ID

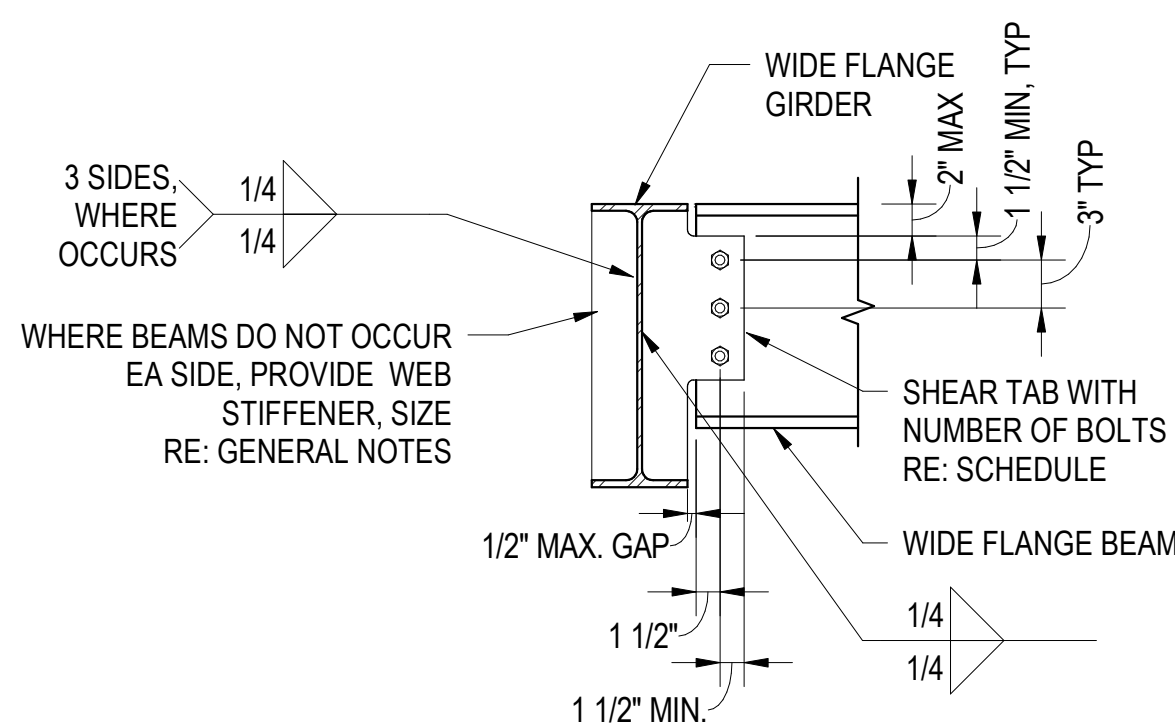
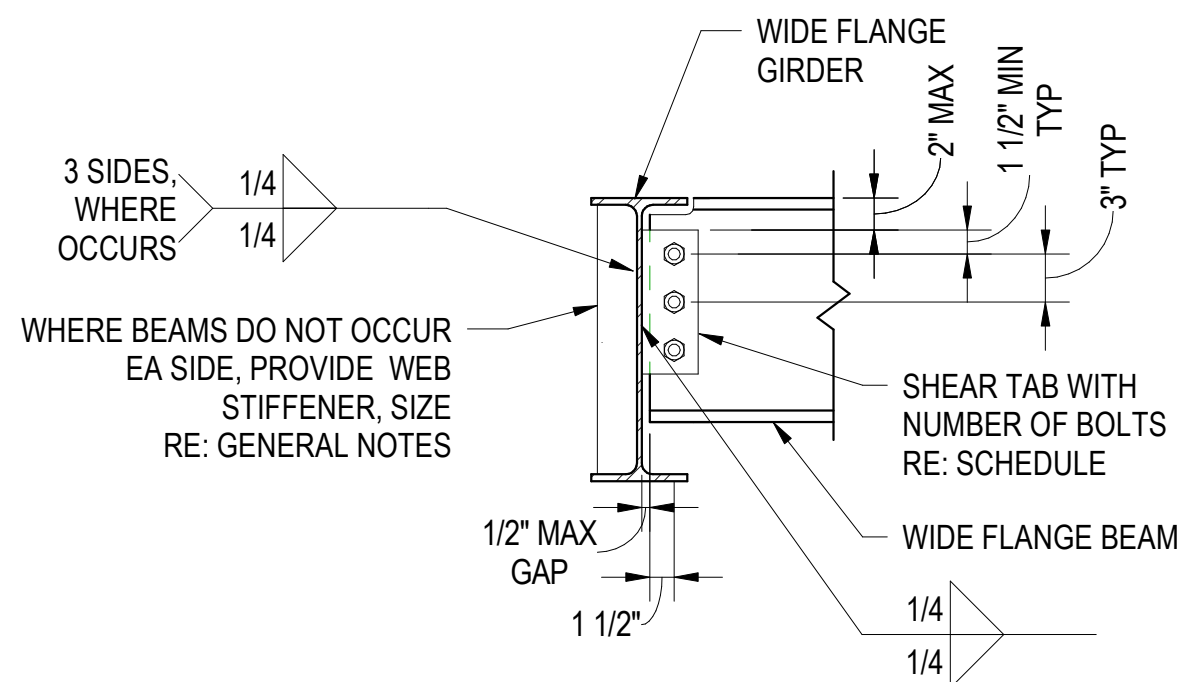
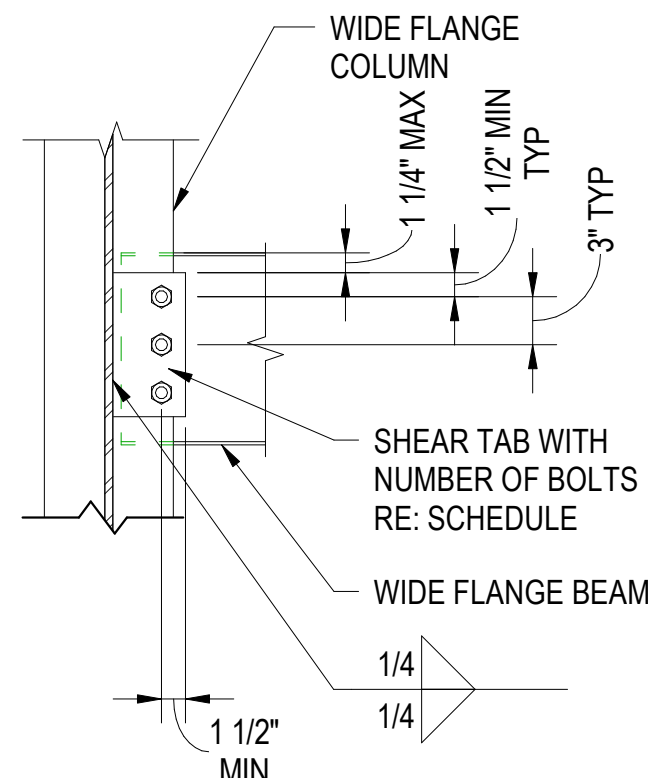
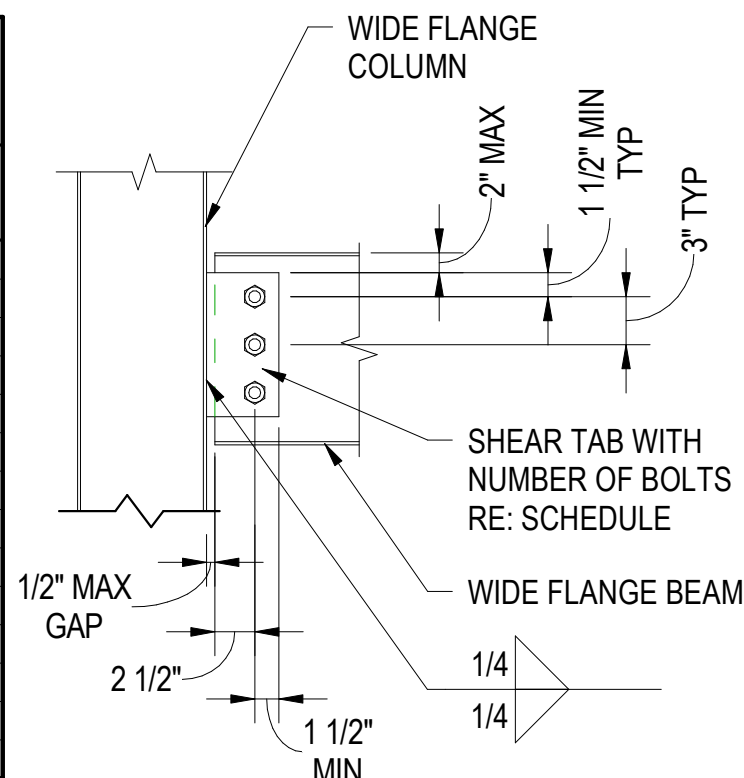
S-602

STEEL COMPOSITE DECK SCHEDULE							
MARK	STEEL DECK			CONCRETE FILL		TOTAL DECK THICKNESS	COMMENTS
	PROFILE	GAUGE	FINISH	WEIGHT	REINF		
SOD1	W2 W/ 4 1/2" NW CONC	20 GA	G60	145 PSF NW	#4 @ 18" OC EA WAY	6 1/2"	

1. STEEL FLOOR DECK MUST COMPLY WITH THE LATEST REQUIREMENTS OF THE STEEL DECK INSTITUTE (SDI).
2. WHEN FIRE PROOFING IS TO BE USED, DECK MUST BE COATED WITH SPECIAL PAINT IN ORDER TO PROPERLY RECEIVE FIRE PROOFING.
3. ALL DECK MUST BE INSTALLED WITH INTERLOCKING SIDE SEAMS, AND MUST BE CRIMPED PRIOR TO CONNECTING.
4. ALL DECK MUST BE 3-SPAN CONTINUOUS MINIMUM. CONTRACTOR MUST CONTACT ENGINEER AND PROVIDE OPTIONS WHEN 3-SPAN IS NOT POSSIBLE.
5. FLOOR DECK MUST BE WELDED TO SUPPORTING FRAME MEMBERS WITH 3/4" DIAMETER PUDDLE WELDS IN A (36/4) TYPE WELD PATTERN AND AT 12" OC AT ALL PERIMETERS AND 12" OC AT OTHER SUPPORTS PARALLEL TO CORRUGATIONS.
6. WHEN STUDS WELD THROUGH THE FLOOR DECK TO THE BEAMS BELOW, THEY MAY TAKE THE PLACE OF DESIGNATED PUDDLE WELDS.
7. ATTACH INTERLOCKING SEAMS WITH BUTTON PUNCH AT 18" OC OR 1 1/2" LONG TOP SEAM WELDS AT 24" OC. SIDE SEAMS MUST BE CRIMPED BEFORE WELDING.
8. MINIMUM DECK BEARING MUST BE 2".
9. ICC REPORT MUST BE SUBMITTED TO SHOW A MINIMUM ALLOWABLE DIAPHRAGM SHEAR VALUE OF XXXX LBS/FT FOR A 8'-0" DECK SPAN.
10. ALL DECK DESIGNATED AS G60 OR G90 IS GALVANIZED DECK.
11. CONCRETE SLAB ON STEEL FLOOR DECK MUST BE REINFORCED AS INDICATED IN THE SCHEDULE. REINFORCING IS TO BE PLACED 1 1/2" BELOW THE TOP OF THE SLAB.
12. GENERAL CONTRACTOR TO FOLLOW MANUFACTURER GUIDELINES FOR ALL DECK, CONNECTION, ATTACHMENTS, ETC.
13. FOR FINAL FINISH SEE ARCHITECT DRAWINGS AND SPECIFICATIONS.
14. RE: PLAN FOR NUMBER AND SIZE OF STUDS REQUIRED AT EACH BEAM. MAXIMUM STUD SPACING AT 12" OC.



BEAM SIZE	NUMBER OF BOLTS
W8	(2) 3/4" DIA
W10	(2) 3/4" DIA
W12	(3) 3/4" DIA
W14	(3) 3/4" DIA
W16	(4) 3/4" DIA
W18	(5) 3/4" DIA
W21	(5) 3/4" DIA
W24	(6) 7/8" DIA
W27	(7) 7/8" DIA
W30	(8) 7/8" DIA
W33	(9) 7/8" DIA
W36	(10) 7/8" DIA
W40	(11) 7/8" DIA
W44	(12) 7/8" DIA

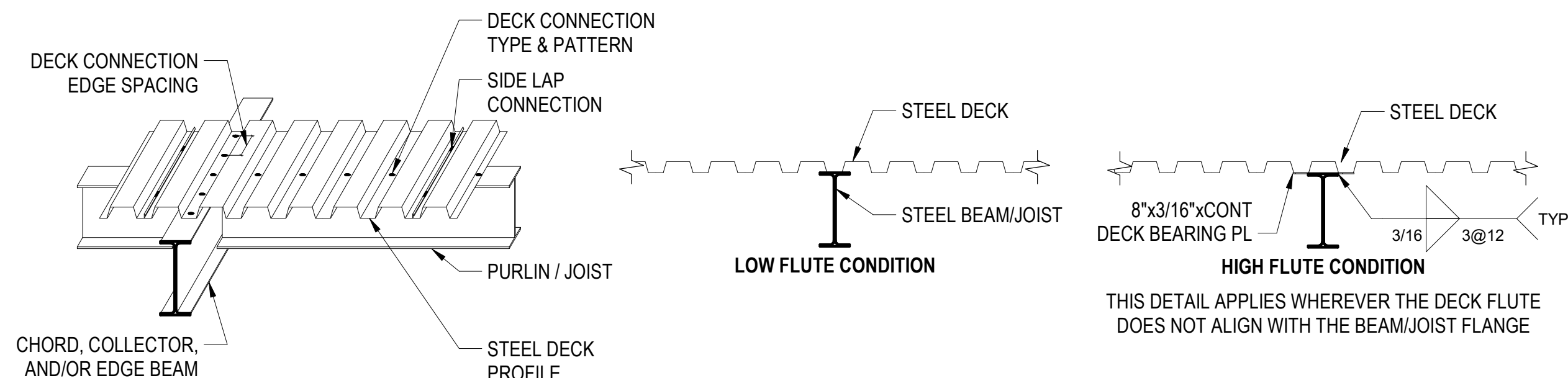


1. BEAM TOP OF STEEL AS SHOWN ON PLAN.
2. SEE PLANS FOR BEAM AND/OR COLUMN SIZES.
3. AT BRACED FRAMES, SEE BRACED FRAME ELEVATIONS/DETAILS FOR ALL STEEL SIZES AND CONNECTIONS.
4. ALL BOLTS SHALL BE A325-N UNLESS NOTED OTHERWISE.
5. ALL SHEAR TABS SHALL BE 3/8" UNLESS NOTED OTHERWISE.
6. DECK NOT SHOWN, SEE PLANS FOR FLOOR AND/OR ROOF STRUCTURE.
7. PROVIDE STANDARD OR HORIZONTAL SHORT SLOTTED HOLES IN SHEAR TAB.
8. FOR SHEAR TABS WELDED TO WIDE FLANGE OR HSS COLUMNS WITH 3/16" WALLS, USE 5/16" SHEAR PLATES.
9. FOR CONNECTIONS DESIGNATED AS DOUBLE-ANGLE CONNECTIONS, USE L5x3 ANGLES ON EACH SIDE OF THE BEAM INSTEAD OF A SHEAR PLATE. FOLLOW THE ANGLE SIZE AND SUPPORTING CONNECTION DETAILS SHOWN ON THE STEEL EMBED SCHEDULE. AT CONTRACTOR'S OPTION, DOUBLE-ANGLE CONNECTIONS CAN BE ALL BOLTED IN LIEU OF WELDING ANGLES TO SUPPORTING MEMBERS AS SHOWN ON THE EMBED SCHEDULE. IF ALL BOLTED, USE THE SAME NUMBER OF 3/4" BOLTS AND SPACING AS SHOWN ON THE SHEAR TABS OF THE STEEL CONNECTION SCHEDULE.

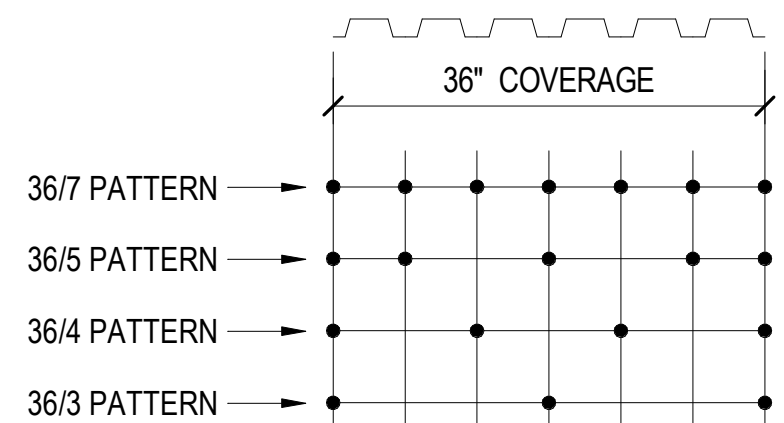
STEEL DECK SCHEDULE										
MARK	STEEL DECK			DECK CONNECTION			SIDE LAP CONNECTION		DECK PERFORMANCE	
	PROFILE	GAUGE	FINISH	TYPE	PATTERN	EDGE SPACING	TYPE	SPACING	MIN SHEAR	MIN FLEXIBILITY
SD1	1 1/2" PLB-36	20	G60	HILTI X-HSN24 PAF	36/7	6" OC	VSC2	8" OC	--	--

1. STEEL ROOF DECK MUST COMPLY WITH THE LATEST REQUIREMENTS OF THE STEEL DECK INSTITUTE (SDI)
2. WHEN FIRE PROOFING IS TO BE USED, DECK MUST BE COATED WITH SPECIAL PAINT IN ORDER TO PROPERLY RECEIVE FIRE PROOFING.
3. ALL DECK MUST BE INSTALLED WITH INTERLOCKING SIDE SEAMS, AND MUST BE CRIMPED PRIOR TO CONNECTING.
4. MINIMUM SHEAR DENOTED IN SCHEDULE REPRESENTS THE MINIMUM ALLOWABLE DECK SHEAR FOR A 6'-0" SPAN REQUIRED FOR APPLICATION IN THIS PROJECT.
5. ALL DECK MUST BE 3-SPAN CONTINUOUS MINIMUM. CONTRACTOR MUST CONTACT ENGINEER AND PROVIDE OPTIONS WHEN 3-SPAN IS NOT POSSIBLE.
6. MINIMUM DECK BEARING MUST BE 2".
7. DO NOT SUPPORT ANY HANGING LOADS FROM STEEL ROOF DECK.
8. ALL DECK DESIGNATED AS G60 OR G90 IS GALVANIZED DECK.
9. GENERAL CONTRACTOR TO FOLLOW MANUFACTURER GUIDELINES FOR ALL DECK, CONNECTION, ATTACHMENTS, ETC.
10. FOR FINAL FINISH SEE ARCHITECT DRAWINGS AND SPECIFICATIONS.

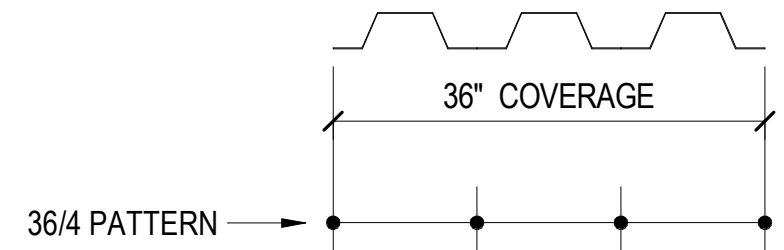
ASW = 3/4" ARC SPOT WELD (PUDDLE WELD)
BTN = BUTTON PUNCH (3/16")
PIN = PIN CONNECTIONS (POWDER ACTUATED FASTENER)
ACCEPTABLE FASTENERS:
HILTI X-HSN-24 @ SUPPORTS 3/16" THROUGH 3/8" THICK
HILTI X-ENP-19 @ SUPPORTS 1/4" THICK AND GREATER
SCR = #10 SCREW
TBD = DECK CONNECTION AND SIDELAP SPACING TO BE
DETERMINED BY CONTRACTOR TO MEET THE MINIMUM
ALLOWABLE SHEAR AND FLEXIBILITY REQUIREMENTS
LISTED IN SCHEDULE. CONTRACTOR TO SUBMIT ICC
REPORT.
TSW = TOP SEAM WELD (1 1/2" IN LENGTH MINIMUM)
VSC2 = VERCO PUNCHLOCK II SYSTEM
VST = VERCO SHEARTRANZ II-42 SYSTEM



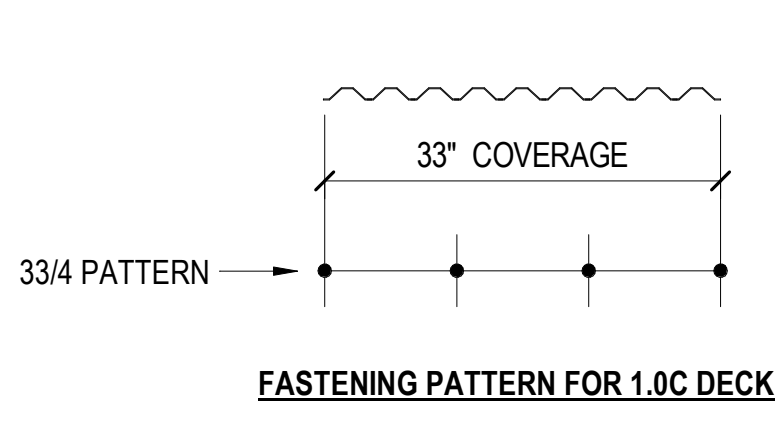
STEEL DECK FASTENING PATTERN TO SUPPORT



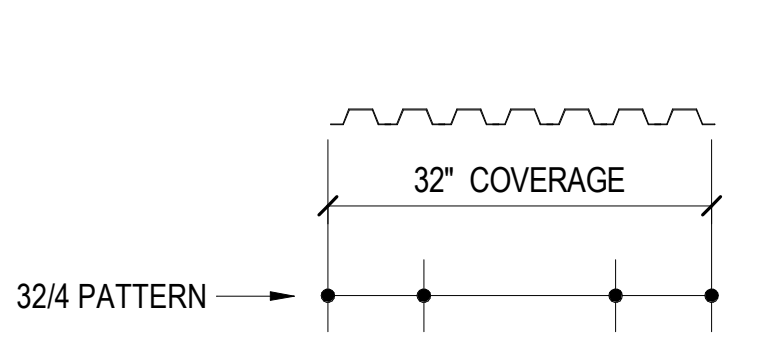
FASTENING PATTERNS FOR 1.5 (B,F,A,VL) DECK



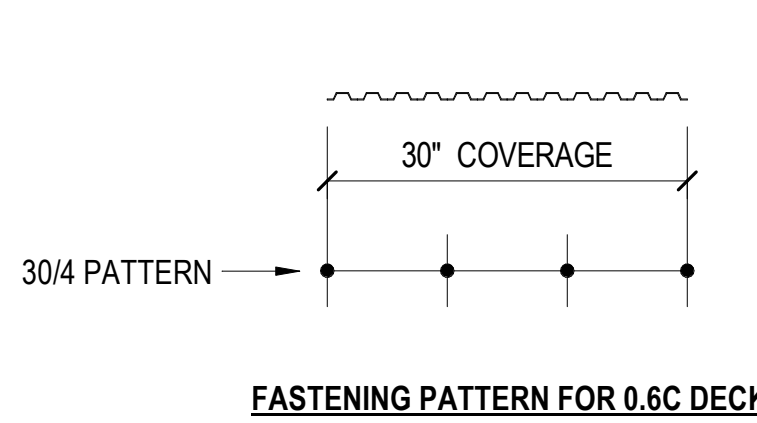
FASTENING PATTERN FOR 2VLI AND 3VLI DECK



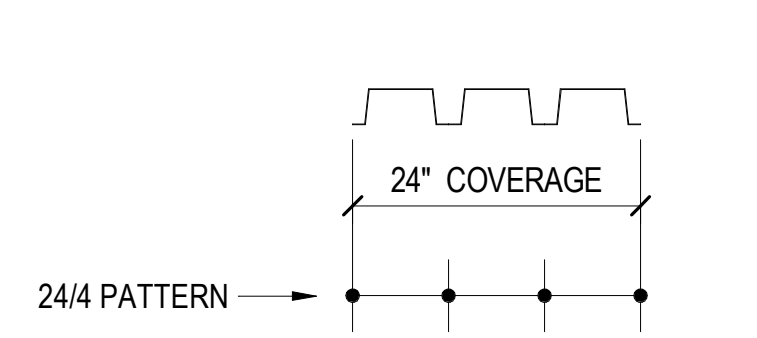
FASTENING PATTERN FOR 1.0C DECK



FASTENING PATTERN FOR 1.3C DECK



FASTENING PATTERN FOR 0.6C DECK



FASTENING PATTERN FOR 3N DECK

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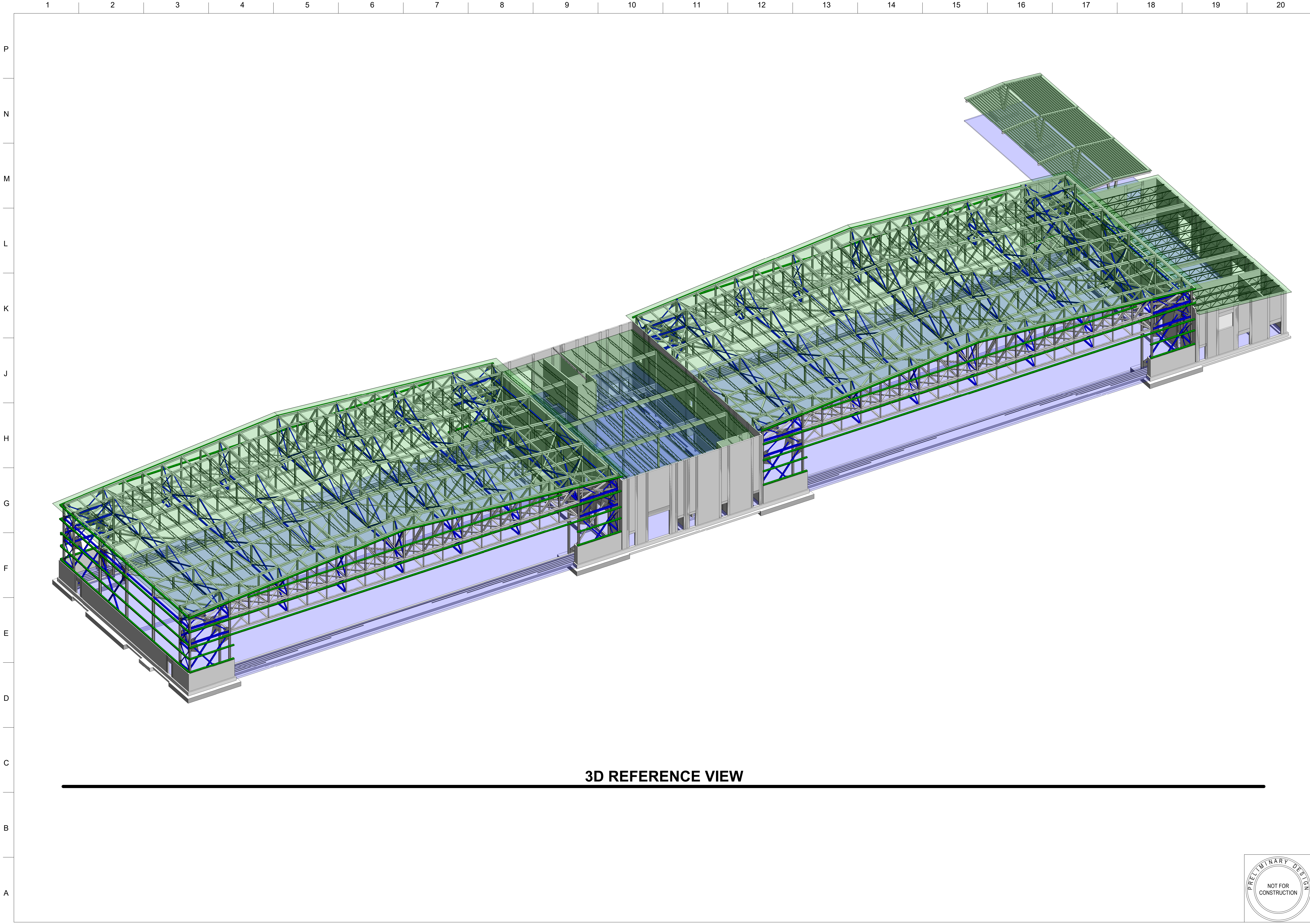
US ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT	DESIGNED BY: J. CARLSON	ISSUE DATE: NOVEMBER 13, 2025
	CHECKED BY: R. CARLSON	CONTRACT NO.: W912PL25C0001
	CHECKED BY: D. CARLSON	CONTRACT NO.: W912PL25C0007
	SUBMITTED BY: MSZCZUK	
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ANSI D SIZE:		

CREECH AIR FORCE BASE, CLARK COUNTY, NV
DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2
494137

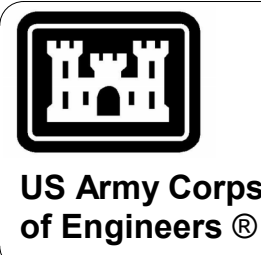
STRUCTURAL SCHEDULES

SHEET ID

S-603



3D REFERENCE VIEW



MARK	DESCRIPTION	DATE

DESIGNED BY: A. VALENCIA	ISSUE DATE: NOVEMBER 13, 2025
DRAWN BY: R. CARLSON	SOLICITATION NO.: W912PL23RA0012
CHECKED BY: D. CLAYSON	CONTRACT NO.: W912PL25C0037
SUBMITTED BY: P. PASZCZUK	
SIZE: ANSI D	

US ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT	KORTE CONSTRUCTION 5700 OAKLAND AVE, SUITE 275 ST. LOUIS, MO 63110
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CREECH AIR FORCE BASE, CLARK COUNTY, NV DISASTER RESILIENCY PROGRAM (DRP) - PHASE 2 494137	3D REFERENCE VIEW
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SHEET ID S-901

